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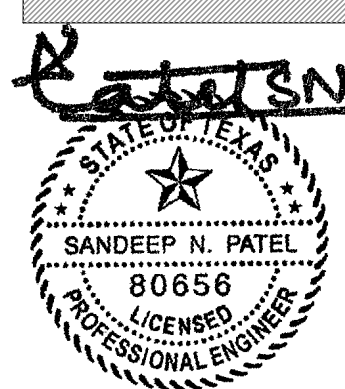
MASONRY/BRICK VENEER WITH CMU BACKING (ADJUSTABLE WIRE):

1. VENEER TIES SHALL BE 3/16" DIA. WIRE LOOP
MINIMUM OF .80 TIMES THE WIDTH OF THE VENEER

[illegible]

Drawn By:	Checked By:
HT	DVH/ZA
Drawing Scale:	Project No.
As Noted	136-091

ISSUED FOR:	DATE:
<input type="checkbox"/> SD 30%	_____
<input type="checkbox"/> Coordination	_____
<input type="checkbox"/> CD 95%	_____
<input type="checkbox"/> CD 100%	_____
<input type="checkbox"/> Pricing	_____
<input type="checkbox"/> Bidding	_____
<input type="checkbox"/> Permit	_____
<input type="checkbox"/> Construction	_____



F-19122
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SHEET NO.

S0-1

GENERAL NOTES

C:\\$136 - KELLY GROSSMAN\136-091 Moonlight Ranch\Structural Drawings\Working Drawings\50-2.dwg Plotted: June 10, 2019 - 11:02 AM by Hao Tran

- 1
- PARALLEL STRAND (PSL) AND GLULAM LUMBER (GLB)
1.

PSL LUMBER SHALL BE FABRICATED FROM LONG, THIN STRANDS OF EITHER EASTERN OR WESTERN SPECIES WOOD BONDED TOGETHER WITH A MICROWAVE PROCESS.

2.

EASTERN PSL LUMBER (ES) MAY INCLUDE SOUTHERN PINE OR YELLOW POPLAR. WESTERN PSL BEAMS (WS) MAY INCLUDE DOUGLAS FIR, LODGEPOLE PINE, WESTERN HEMLOCK OR WHITE FIR.

3.

PSL LUMBER SHALL BE FABRICATED IN PARALLEL STRANDS (PSL) IN CONFORMANCE WITH NER 292.

4.

GLB LUMBER SHALL BE FABRICATED FROM LAMINATED 2x LUMBER ACCORDING TO STANDARDS SET FORTH IN NDS AND OTHER APPLICABLE CODES.

5.

THE MEMBERS SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN STRESSES:

	PSL	GLB
a. SHEAR MODULUS OF ELASTICITY (G)	125,000 PSI	125000 PSI
b. MODULUS OF ELASTICITY (E)	2,0X10 ⁶ PSI	2.0 X 10 ⁶ PSI
	2,900 PSI	24,00 PSI
c. COMPRESSION PERPENDICULAR TO GRAIN AND PARALLEL TO WIDE FACE OF STRANDS (fc)	750 PSI	740 PSI
d. COMPRESSION PERPENDICULAR TO GRAIN AND PERPENDICULAR TO WIDE FACE OF STRANDS (fc)	525 PSI	525 PSI
e. COMPRESSION PARALLEL TO GRAIN (fc)	2,900 PSI	2,400 PSI
f. HORIZONTAL SHEAR PERPENDICULAR TO WIDE FACE OF STRANDS (fv)	290 PSI	290 PSI
g. HORIZONTAL SHEAR PARALLEL TO WIDE 210 PSI FACE OF STRANDS (fv)	210 PSI	210 PSI

6.

HEAL CUTS ON BEAMS MUST NOT OVERHANG INSIDE FACE OF SUPPORT MEMBER.

7.

PSL LUMBER MEMBERS SHALL BE FABRICATED WITHOUT CAMBER. GLULAM BEAMS MAY BE CAMBERED TO REMOVE DEAD LOAD DEFLECTION.

8.

THE PSL AND GLB MEMBERS SHALL BE PROTECTED FROM THE WEATHER WHILE IN STORAGE. CARE SHALL BE EXERCISED DURING HANDLING TO PREVENT DAMAGE.

- 2
- CONVENTIONAL 2X WOOD FRAMING
1.

LUMBER AND ITS FASTENINGS, SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATIONS OF STRESS-GRADE LUMBER AND ITS FASTENINGS, LATEST EDITION, AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
2.

MATERIALS FOR EXTERIOR WALLS, INTERIOR BEARING WALLS AND SHEARWALLS SHALL BE NO. 2 SOUTHERN YELLOW PINE (MC19) OR BETTER AND SHALL HAVE THE FOLLOWING UNFACTORED MINIMUM ALLOWABLE DESIGN STRESSES.

	2x4	2x6
a. MODULUS OF ELASTICITY (E)	1.4 X 10 ⁶ PSI	1.4 X 10 ⁶ PSI
b. MIN. MODULUS OF ELASTICITY (EMIN)	0.51 X 10 ⁶ PSI	0.51 X 10 ⁶ PSI
c. FLEXURAL STRESS (FB)	1,100 PSI	1,000 PSI
d. COMPRESSION PERPENDICULAR TO GRAIN (FC)	565 PSI	565 PSI
e. COMPRESSION PARALLEL TO GRAIN (FC)	1,450 PSI	1,400 PSI
f. SHEAR PARALLEL TO GRAIN (FV)	175 PSI	175 PSI
g. TENSION PARALLEL TO GRAIN (FT)	675 PSI	600 PSI
3.

LUMBER FOR HEADERS, BEAMS, AND OTHER FRAMING MEMBERS SHALL BE #2 SYP (MC19) OR BETTER
4.

LOAD BEARING WALLS, CONSTRUCTED FROM FINGER JOINTED STUDS SHALL BE SHEATHED ON AT LEAST ONE FACE OR BRACED W/ 1x4 HORIZONTAL (CONT.) AT MID-HEIGHT OF WALL.
5.

FINGER JOINTED STUDS SHALL MEET OR EXCEED THE MATERIAL PROPERTIES AND ALLOWABLE STRESSES FOR SOLID LUMBER AS SPECIFIED FOR STUD GRADE CONSTRUCTION.
6.

TOP AND BOTTOM PLATES SHALL BE 2x4 #2 OR 2x6 #2 SOUTHERN YELLOW PINE(MC19).
7.

SUBJECT TO ENGINEER'S REVIEW AND ACCEPTANCE, OTHER WALL CONSTRUCTION SHALL BE EITHER CONSTRUCTION GRADE OR UTILITY HEADER AND OTHER MISCELLANEOUS FLEXURAL MEMBERS SHALL BE NO. 2 SYP (MC19 OR BETTER U.N.O.)
8.

SUBJECT TO ENGINEER'S REVIEW AND ACCEPTANCE, OTHER NON-STRUCTURAL WALL CONSTRUCTION SHALL BE EITHER CONSTRUCTION GRADE OR UTILITY SOUTHERN YELLOW PINE (MC19) OR DOUGLAS FIR LARCH (MC19).
9.

MATERIALS MUST BE GRADE MARKED.
10.

SOLE PLATES AT FIRST FLOOR SHALL BE PRESSURE TREATED LUMBER, 0.25ACQ/MCQ OR BORATE (DOT .28) MINIMUM, 2x4 #2 OR 2x6 #2 SOUTHERN YELLOW PINE.
11.

FOR OVERLAY FRAMING AT ROOFS OR OTHER CONVENTIONAL ROOF FRAMING, CONTRACTOR SHALL PROVIDE 2x FRAMING IN ACCORDANCE WITH ROOF RAFTER TABLES IN THE APPLICABLE BUILDING CODE.
12.

BOLT HOLES THROUGH WOOD SHALL BE DRILLED 1/16" MAXIMUM LARGER THAN THE DIAMETER OF THE BOLTS TO BE INSTALLED.
13.

BOLTS THROUGH WOOD SHALL BE FITTED WITH STANDARD WASHERS AT HEAD AND NUT ENDS.
14.

FLITCH BEAMS AND MULTIPLE MEMBER WOOD BEAMS WHEN SHOWN ON PLANS SHALL BE BOLTED TOGETHER WITH ONE 1/2" DIA. BOLT, TOP AND BOTTOM OVER THE SUPPORTS AND/OR AT THE ENDS OF THE BEAM AND 24" ON CENTER, STAGGERED FULL LENGTH OF THE BEAM.
15.

A HOLE GREATER IN DIAMETER THAN 40 PERCENT OF THE STUD WIDTH MAY NOT BE BORED IN ANY WOOD STUD. BORED HOLES IN DIAMETER EQUAL TO 60 PERCENT OF THE WIDTH OF THE STUD ARE PERMITTED IN NON-LOAD BEARING PARTITIONS OR WALLS WHERE EACH BORED STUD IS DOUBLED, PROVIDED NOT MORE THAN TWO SUCH SUCCESSIVE DOUBLE STUDS OCCUR.
16.

EDGE OF A BORED HOLE SHALL NOT BE WITHIN 5/8 INCH OF THE STUD EDGE. BORED HOLES SHALL NOT BE LOCATED AT A CUT OR NOTCH IN THE STUD.

- 3
- WOOD FRAMING TOLERANCES FOR SHRINKAGE
1.

THE CONSTRUCTION OF A 3-STORY, TYPE FIVE WOOD FRAME REQUIRES AN UNDERSTANDING OF FRAMING TOLERANCES, SHRINKAGE, INTERACTION WITH DISSIMILAR MATERIALS AND CONTRACTOR SHOULD DEVELOP A PROACTIVE QUALITY CONTROL PROCEDURE AND REVIEW WITH ARCHITECT AND ENGINEER.
2.

THE APPROXIMATE SHRINKAGE IN THE CONVENTIONAL 2x WOOD FRAME IS AS FOLLOWS: THREE STORY.....3/4" TO 1/4".
3.

ROUGH OPENINGS IN EXTERIOR WALLS SHALL BE UPSIZED APPROXIMATELY 1 1/2" TO ACCOMMODATE SHRINKAGE PRIMARILY AT TOP FLOORS.
4.

PROVIDE 1/8" WIDE JOINT IN SHEATHING AT TOP AND BOTTOM OF THE FLOOR CAVITY AT EACH LEVEL.

- 4
- FOUNDATIONS ON EXPANSIVE SOIL
- DESIGN ASSUMPTIONS:

1.

SITE SLOPE SHALL CAUSE WATER TO FLOW AWAY FROM THE BUILDING FOOTPRINT FOR A MINIMUM DISTANCE OF 10 FT.

2.

NO VEGETATION OVER SIX FEET IN HEIGHT SHALL BE PLANTED WITHIN 20 FEET OF BUILDING PERIMETER UNLESS SPECIALLY COUNTED FOR.

3.

THE DESIGN ASSUMES DOWN-SPROUTS TO BE TIED INTO STORM-DRAIN OR OTHER MEANS TO DIRECT EXCESSIVE LOCALIZED MOISTURE AWAY FROM THE BUILDING FOOTPRINT.

4.

IT IS ASSUMED THAT THE SITE WILL BE MAINTAINED DURING ITS USEFUL LIFE-CYCLE OF POST-TENSIONED SLAB.

5.

IT IS NOT RECOMMENDED TO DESIGN OR CONSTRUCT A POST-TENSION SLAB OVER AN AREA COVERING PARTIALLY CUT AND FILL OF EXPANSIVE OR COMPRESSIBLE SOILS, WITHOUT DUE SETTLEMENT CONSIDERATIONS.

6.

UNLESS OTHERWISE NOTED ON THE PLANS: STOOPS, ELECTRICAL/MECHANICAL PADS, PORCHES AND PATIOS OR OTHER ATTACHMENTS SHALL BE DESIGNED AND CAST INDEPENDENTLY OF THE POST-TENSIONED SLAB FOUNDATION. THE DESIGN OF SUCH ADDITIONS SHALL BE SUBMITTED TO SECOG FOR APPROVAL TO ENSURE THAT IT DOES NOT ADVERSELY AFFECT THE PERFORMANCE OF THE BUILDING SLAB. EXCEPTION IS TAKEN FOR FOUNDATION EXTENSIONS SUPPORTING THE SUPERSTRUCTURE (SUCH AS POSTS) OR SUPPORTING MEMBERS WHICH ARE CONNECTED TO THE BUILDING. SUCH SLAB EXTENSIONS SHALL BE PART OF THE SLAB.

COMPATIBILITY CONSIDERATIONS:

1.
- POST-TENSIONED SLAB FOUNDATIONS AS WELL AS OTHER CONVENTIONALLY REINFORCED SHALLOW FOUNDATION SYSTEMS CONSTRUCTED ON COMPRESSIBLE OR EXPANSIVE SOILS ARE EXPECTED TO DEFORM. THE FLEXIBLE FOUNDATION PRIMARILY DISTRIBUTES LOCALIZED SOIL MOVEMENT TO A MORE UNIFORM SLAB SHAPE (EDGE LIFT, CENTER LIFT). OTHER CONSULTANTS AND SUPPLIERS SHALL CONSIDER COMPATIBLE DESIGNS ORPRODUCTS FOR THE SELECTED FLEXIBLE FOUNDATION SYSTEM. IN PARTICULAR, DEFORMATION COMPATIBILITY SHOULD BE ADDRESSED FOR ROOF TRUSSES, LOAD CONCENTRATION, BRITTLE EXTERIOR SIDING, AREAS WHICH SLOPE TO DRAIN AND UTILITY CONNECTIONS. THE FOUNDATION IS INTENDED TO MOVE WITHIN THE SPECIFIED SERVICEABILITY LIMITS AS FOLLOWS:

MATERIAL	CENTER LIFT	EDGE LIFT
WOOD FRAME	L/240	L/480
STUCCO OR PLASTER	L/360	L/720
BRICK VENEER	L/480	L/960
CONCRETE MASONRY UNITS	L/960	L/1920
PREFAB ROOF TRUSSES*	L/1000	L/2000

*TRUSSES WHICH CLEARSAN THE FULL LENGTH OR WIDTH OF THE FOUNDATION FROM EDGE TO EDGE.

SOIL PARAMETERS:

1.
- REFER TO THE COVER SHEET FOR ALL OF THE DESIGN VALUES USED TO DESIGN THE POST-TENSION FOUNDATIONS. ALL OF THESE VALUES ARE BASED ON THE DESIGN VALUES GIVEN BY THE SPECIFIED GEOTECHNICAL ENGINEER.
2.
- FACORS NOT RELATED TO CLIMATE MAY INDUCE SOIL MOVEMENTS MANY TIMES LARGER THAN THOSE RESULTING FROM CLIMATE INFLUENCES ALONE. WHILE IT MAY BE POSSIBLE TO QUANTIFY THE EFFECTS OF MANY NON-CLIMATIC FACTORS, THEIR PRESENCE OR ABSENCE IS OFTEN BEYOND THE DIRECT CONTROL OF THE STRUCTURAL AND/OR GEOTECHNICAL ENGINEER. IN GENERAL, AN EFFECTIVE MEANS FOR MITIGATING NON-CLIMATIC FACTORS IS TO PROVIDE DETAILED LIMITATIONS ON CONSTRUCTION AND USE ON THE PLANS AND/OR CONTRACT DOCUMENTS. SOME DESIGNERS AND BUILDERS ACTUALLY PREPARE "USER MANUALS" FOR THE OWNER OF BUILDINGS ON EXPANSIVE SOILS, WITH DETAILED GUIDELINES ON IRRIGATION, DRAINAGE, VEGETATION, SLOPES, AND OTHER NON-CLIMATIC FACTORS WHICH MAY AFFECT THE PERFORMANCE OF THE FOUNDATION. THE MAJOR FACTORS INFLUENCING SOIL MOVEMENT THAT ARE NOT RELATED TO CLIMATE ARE:

(A) PRE-VEGETATION:

LARGE INDIVIDUAL TREES, THICKETS OR OTHER VEGETATION REQUIRING LARGE AMOUNTS OF MOISTURE FROM THE SOIL TEND TO MAKE THE SOIL IN THE AREAS REACHED BY THEIR ROOTS DRIER THAN ADJACENT AREAS. THESE DESICCATED POCKETS HAVE A MUCH HIGHER POTENTIAL FOR SWELLING THAN DO THE ADJACENT, LESS DESICCATED AREAS.

(B) FENCE LINES, TRAILS, AND TRACKS:

THESE SURFACE FEATURES TYPICALLY HAVE THE VEGETATION WORN AWAY, LEAVING ONLY BARE OR THINLY COVERED STRIPS WHICH ARE MUCH DRIER THAN THE SOIL ON EITHER SIDE. LIKE THE DESICCATED AREAS CAUSED BY PRE-CONSTRUCTION VEGETATION, THESE AREAS WILL SWELL MORE THAN OTHER AREAS.

(C) SLOPES:

SLOPES COMPRISED OF ACTIVE EXPANSIVE SOIL HAVE A TENDENCY TO MIGRATE DOWNHILL AS THE SOIL EXPERIENCES SHRINK-SWELL CYCLES.

(D) CUT AND FILL SECTIONS:

CUT AND FILL SECTIONS WILL EXPERIENCE DIFFERENTIAL SOIL MOVEMENT BECAUSE OF VARIATIONS OF COMPACTED DENSITIES.

(E) DRAINAGE:

IF RAINFALL RUNOFF IS ALLOWED TO POND OR COLLECT ADJACENT TO A STRUCTURE BUILT ON EXPANSIVE SOIL, THE STRUCTURE MAY BE SUBJECTED TO DISTRESS CAUSED BY THE SOIL BENEATH THE STRUCTURE SWELLING AS A DIRECT RESULT OF INCREASED SOIL MOISTURE CONTENT. LOT SURFACES MUST BE GRADED TO DRAIN AWAY FROM THE STRUCTURE. EXCESS RUNOFF SHOULD NOT BE COLLECTED AND DISPOSED OF BY CARRYING DISCHARGE PIPE BENEATH THE STRUCTURE. CARE SHOULD ALSO BE TAKEN WITH SEWAGE AND WATER UTILITY LINES TO ENSURE THAT LEAKS DO NOT DEVELOP BENEATH THE SLAB.

(F) TIME OF CONSTRUCTION:

IF THE SLAB IS CAST AT THE END OF A LENGTHY DRY PERIOD, IT MAY EXPERIENCE GREATER UPLIFT AROUND THE EDGES WHEN THE SOIL BECOMES WETTER AT THE CONCLUSION OF THE DRY PERIOD. SIMILARLY, A SLAB CAST AT THE END OF A WET PERIOD, MAY EXPERIENCE GREATER DRYING AROUND THE EDGES DURING THE SUBSEQUENT PERIOD OF DRYNESS.

(G) POST-CONSTRUCTION:

A NUMBER OF POST-CONSTRUCTION PRACTICES BEYOND THE CONTROL OF THE DESIGN ENGINEER CAN OCCUR TO CAUSE DISTRESS TO STRUCTURES FOUNDED ON EXPANSIVE CLAY. PLANTING FLOWER BEDS OR SHRUBS NEXT TO THE FOUNDATION AND KEEPING THESE AREAS FLOODED WILL GENERALLY CAUSE A NET INCREASE IN SOIL EXPANSION AROUND THE FOUNDATION PERIMETER IN THAT VICINITY. PLANTING SHADE TREES CLOSER TO THE STRUCTURE THAN A DISTANCE TO HALF THE MATURE HEIGHT OF THE TREE WILL ALLOW THE TREE ROOTS TO PENETRATE BENEATH THE FOUNDATION AND WITHDRAW MOISTURE FROM THE SOIL. THE RESULT WILL BE A SOIL SHRINKAGE IN THE REGION OF THE ROOTS, REDIRECTING SURFACE RUNOFF CHANNELS OR SWALES BY THE OWNER CAN RESULT IN IMPROPER DRAINAGE AS DETAILED ABOVE. TO MINIMIZE MOVEMENTS IN SOILS DUE TO POST - CONSTRUCTION FACTORS THAT ARE NOT CLIMATE RELATED, THE FOLLOWING OWNERS MAINTENANCE PROCEDURES ARE RECOMMENDED:

(i)

INITIAL LANDSCAPING SHOULD BE DONE ON ALL SIDES ADJACENT TO THE FOUNDATION AND DRAINAGE AWAY FROM THE FOUNDATION SHOULD BE PROVIDED.

(ii)

WATERING SHOULD BE DONE IN A UNIFORM, SYSTEMATIC MANNER AS POSSIBLE ON ALL SIDES OF THE FOUNDATION TO KEEP THE SOIL MOIST. AREAS OF SOIL THAT DO NOT HAVE GROUND COVER MAY REQUIRE MORE MOISTURE AS THEY ARE MORE SUSCEPTIBLE TO EVAPORATION. PONDING OR TRAPPING OF WATER IN LOCALIZED AREAS ADJACENT TO THE FOUNDATIONS CAN CAUSE DIFFERENTIAL MOISTURE LEVELS IN SUBSURFACE SOILS.

(iii)

STUDIES HAVE SHOWN THAT TREES WITHIN 20 FEET OF FOUNDATIONS HAVE CAUSED DIFFERENTIAL MOVEMENTS IN FOUNDATIONS. THESE WILL REQUIRE MORE WATER IN PERIODS OF EXTREME DROUGHT AND IN SOME CASES A ROOT INJECTION SYSTEM MAY BE REQUIRED TO MAINTAIN MOISTURE EQUILIBRIUM.

(iv)

DURING EXTREME HOT AND DRY PERIODS, CLOSE OBSERVATIONS SHOULD BE MADE AROUND FOUNDATIONS TO INSURE THAT ADEQUATE WATERING IS BEING PROVIDED TO KEEP SOIL FROM SEPARATING OR PULLING BACK FROM THE FOUNDATION.

- 5
- METALS
- 5.01 -- HANDRAILS

1.

HANDRAILS AND GUARD SHALL BE DESIGNED IN COMPLIANCE WITH IBC 1607.7.1, TO RESIST A 50 PLF LIVE LOAD ALONG ITS TOP EDGE AND A NON-CONCURREANT 200# CONCENTRATED LOAD, APPLIED IN ANY DIRECTION. DEFERRED SUBMITTAL SHALL BE SUPPLIED FOR PRE-FABRICATED METAL STAIRS AND STEEL CANOPIES.
- 5.02 -- STEEL STAIRS

1.

ALL STAIRS ARE TO BE STEEL STAIRS PER THE ARCHITECTURAL DRAWINGS EXCEPT WHERE CONCRETE STAIRS ARE SPECIFICALLY SHOWN ON THE DRAWINGS.

2.

STRUCTURAL CALCULATIONS AND DRAWINGS SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE ARE TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

3.

DRAWINGS AND BIDS ARE TO INCLUDE CONNECTIONS TO THE STRUCTURE.

4.

ENGINEER WILL APPROVE THE DRAWINGS AS TO THEIR COMPLIANCE WITH THE INTENT OF THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.

5.

ALL STAIRS MUST BE DESIGNED FOR LIVE AND DEAD LOADS PER CODE.

6.

STAIR SUPPORTS MAY BE HUNG FROM FLOOR FRAMING OR SELF-SUPPORTED.

7.

STAIR FABRICATOR SHALL COORDINATE CONSTRUCTION OF STAIRS WITH GENERAL CONTRACTOR.

8.

THE STAIR FRAMING AND CONNECTIONS DETAILING, INCLUDING TREADS, STRINGERS, LANDINGS AND HANDRAILS SHALL BE SUBMITTED TO ENGINEER FOR HIS REVIEW. THE STAIR SHOP DRAWING WITH SUPPORTING DESIGN SHALL BE SEALED BY A REGISTERED ENGINEER.
-
- STERLING ENGINEERING
DESIGN GROUP
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P.(281)583-7088 F.(281)583-5495
- MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]
- GENERAL NOTES
- Sheet Title
- Date
- Description
- Rev.
- Drawn By: HT
Checked By: DWH/ZA
Drawing Scale: As Noted
Project No. 136-091
- ISSUED FOR: DATE:
☐ SD 30%
☐ Coordination
☐ CD 95%
☐ CD 100%
☐ Pricing
☐ Bidding
☐ Permit
☐ Construction
-
- 06/10/2019
Texas Registered Engineering Firm
F-19122
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- SHEET NO.
- S0-2
- GENERAL NOTES

LENGTH OF SHEARWALL / NO. OF SHEAR PANELS													
WALL TYPE	8'	10'	12'	14'	16'	18'	20'	22'	24'	26'	28'	30'	
[2 -]	1	2	2	2	2	2	3	3	3	3	3	4	
[2 1]	2	2	2	3	3	3	4	4	4	4	5	5	
[2 3]	2	3	3	4	4	5	5	5	6	6	7	7	
[3 -]	2	2	2	2	2	2	2	3	3	3	3	3	
[3 1]	2	2	3	3	3	4	4	5	5	5	6	6	
[3 3]	2	2	3	3	4	4	5	5	5	5	6	6	
[5 -]	2	3	3	4	4	5	5	6	6	7	7	8	
[5 1]	3	3	4	5	6	6	7	8	9	9	10	11	
[5 3]	3	4	5	6	7	7	8	9	10	11	12	13	
[5 5]	5	6	7	8	10	11	12	13	15	16	17	19	
[6 -]	4	5	5	6	7	8	9	9	10	11	12	13	
[6 1]	5	6	7	8	9	10	12	13	14	15	16	17	

- NOTES:
- SHEAR PANELS ARE PREFABRICATED COMPONENTS INSTALLED IN THE FLOOR CAVITY WHEN FRAMING IS PERPENDICULAR TO SHEARWALL.
 - SHEAR PANELS TRANSMIT THE DIAPHRAGM SHEARS FROM THE DIAPHRAGM ABOVE TO THE WALL BELOW.
 - SCHEDULE SHALL BE USED WHEN PANEL QUANTITIES ARE NOT SHOWN ON BRACING PLANS.
 - PANELS SHALL BE DESIGNED FOR A LATERAL FORCE OF 1200 LBS.

1 SHEAR PANEL SCHEDULE

BEAM SIZE (INCHES x INCHES)	TRIBUTARY FLOOR AREA			NOTES
	<50FT ²	<100FT ²	<150FT ²	
(2) - 2x10	HGUS210-2	N/A	N/A	1 & 2
(2) - 2x12	HGUS212-2	N/A	N/A	1 & 2
(3) - 2x10	HGUS210-3	N/A	N/A	1 & 2
(3) - 2x13	HGUS212-3	N/A	N/A	1 & 2
3 1/2 x 9 1/4 PSL	HGUS48	HGUS48	N/A	1 & 2
3 1/2 x 11 1/4 PSL	HGUS48	HGUS48	N/A	1 & 2
3 1/2 x 14 PSL	HGUS410	HGUS410	HGUS412	1 & 2
5 1/4 x 14 PSL	N/A	HGUS5.5/10	HGUS5.5/12	1 & 2
5 1/4 x 16 PSL	N/A	HGUS5.5/10	HGUS5.5/14	1 & 2
5 1/4 x 18 PSL	N/A	HGUS5.5/12	HGUS5.5/14	1 & 2

- NOTES:
- HANGERS SHALL BE AS MANUFACTURED BY SIMPSON STRONG-TIE.
 - ALTERNATES TO THE SIZES SHOWN ABOVE MAY BE SUBMITTED TO THE ENGINEER FOR APPROVAL ALONG WITH PERTINENT TECHNICAL DATA.
 - TRUSS TO BEAM HANGERS SHALL BE DESIGNED BY A TRUSS DESIGN ENGINEER.

5 BEAM TO BEAM CONNECTOR SCHEDULE

EPOXY-EMBEDDED HOLD DOWN ANCHOR BOLT SCHEDULE					
MARK	SIMPSON HOLD DOWN	ANCHOR BOLT DIAMETER	EMBEDMENT DEPTH	SIMPSON THREADED ROD ANCHOR	END MEMBER
HLD-1	LIT19	5/8"	2 1/2"	A307 (SAE 1018)	4x4 OR DBL 2x
HLD-2	HTT4	5/8"	3 3/4"	A307 (SAE 1018)	4x4 OR DBL 2x
HLD-3	HTT5	5/8"	5"	A307 (SAE 1018)	4x4 OR DBL 2x
HLD-4	HD7B	7/8"	6"	A307 (SAE 1018)	4x4 OR DBL 2x
HLD-5	HD9B	7/8"	6"	A307 (SAE 1018)	4x6 OR TPL 2x
HLD-6	HD9B	7/8"	7 3/4"	A307 (SAE 1018)	4x6 OR TPL 2x
HLD-7	HD12	1"	9"	A307 (SAE 1018)	4x6 OR TPL 2x
HLD-8	HD12	1"	12"	A307 (SAE 1018)	4x6

- NOTES:
- ALL THREADED ROD ANCHORS GIVEN IN THIS TABLE SHALL BE INSTALLED WITH SIMPSON SET-XP EPOXY OR APPROVED EQUAL.
 - THE LOAD VALUES OF THESE ANCHOR BOLTS AND HOLD DOWNS HAVE BEEN INCREASED BY 33% FOR WIND LOADS, IN COMPLIANCE WITH THE CODE REQUIREMENTS.
 - FOR INSTALLATION PROCEDURE OF THREADED ROD ANCHORS, SEE ANCHORING SYSTEMS IN SIMPSON CATALOGUE.
 - ALL HARDWARE IN CONTACT WITH ACQ TREATED LUMBER CLASS G185 MUST BE SIMPSON ZMAX PRODUCTS THAT MEET ASTM A653.

10 SHEARWALL HOLD DOWN SCHEDULE

THREE STORY BUILDING						
LEVEL	LOAD BEARING WALLS (SYP #2 GRADE)					
	PLATE HT.	EXTERIOR	INTERIOR	PARTY	CORRIDOR	
					(TYP.)	SUPPORTING FT & CT OR OVER 6' SPAN
3rd FLOOR	9'-1 1/8"	2x4@16"O.C.*	2x6@16"O.C.*	2x4@16"O.C./2x6@16"O.C.	2x4@16"O.C./2x6@16"O.C.	2x4@16"O.C./2x6@16"O.C.
2nd FLOOR	9'-1 1/8"	(2)2x4@16"O.C.*	2x6@16"O.C.*	2x4@12"O.C./2x6@16"O.C.	2x4@16"O.C./2x6@16"O.C.	(2)2x4@16"O.C./2x6@16"O.C.
1st FLOOR	9'-1 1/8"	(2)2x4@16"O.C.*	2x6@16"O.C.*	(2)2x4@16"O.C./2x6@16"O.C.	(2)2x4@16"O.C./2x6@16"O.C.	(2)2x4@12"O.C./2x6@16"O.C.

- * (2) 2x4 @ 16" O.C. AT EXTREME CORNERS OF STRUCTURE FOR A DISTANCE OF 10 FT. FROM CORNER EACH WAY.
NON LOAD-BEARING WALLS ARE TO BE 2x4 @ 16" O.C. TO MEET UL REQUIREMENTS.

- NOTES:
- STUDS USED IN SHEARWALL FRAMING SHALL BE AS SHOWN IN THE ABOVE SCHEDULE FOR BEARINGS WALLS WITH HEIGHTS LESS THAN OR EQUAL TO PLATE HEIGHT SHOWN ON PLANS.
 - REFER TO "CONVENTIONAL 2X WOOD FRAMING" SECTION OF GENERAL NOTES SHEET FOR THE USE OF FINGER JOINTED STUDS.
 - REFER TO "CONVENTIONAL 2X WOOD FRAMING NOTES" SECTIONS OF GENERAL NOTES FOR MATERIAL SPECIFICATION AND LUMBER GRADE.
 - STUD SPACING SHOWN ARE ON CENTER DIMENSIONS AND ARE BASED ON A MAXIMUM STUD HEIGHT OF 8'-8 5/8" UNLESS OTHERWISE NOTED.

14 WALL STUD SCHEDULE

DESIGN CRITERIA					
	TRUSSES	FLOOR	PRIVATE BALCONY	PUBLIC AREA	ROOF
TCLL	40	40	100	20	
TCOL	20	25	25	15	
BCCL	0	0	0	0	
BCOL	5	5	5	5	
TOTAL	65PSF	70PSF	130PSF	40PSF	
U.S.I.	0	0	0	25%	
SPACING	24" MAX.	24" MAX.	24" MAX.	24" MAX.	
DPH/PITCH	18"	RE: ARCH	RE: ARCH	RE: ARCH	

- NOTES:
- TC = TOP CHORD, BC = BOTTOM CHORD, LL = LIVE LOAD, DL = DEAD LOAD.
 - ROOF LIVE LOADS MAY BE REDUCED ACCORDING TO APPLICABLE CODE REQUIREMENTS FOR RISE AND TRIBUTARY CONSIDERATIONS.
 - LOADS MARKED THUS (*) INCLUDE DISTRIBUTED WEIGHT OF SPRINKLER SYSTEM AT 2 PSF.
 - DEAD LOADS DO NOT INCLUDE SELF WEIGHT.
 - TRUSS MANUFACTURER SHALL BE RESPONSIBLE WITH COORDINATING THE EQUIPMENT LOAD, PENETRATION, DUCTS AND CLEARANCES REQUIREMENTS OF ALL OTHER SYSTEMS INCLUDING MEP PRESENT IN THE FLOOR AND ROOF CAVITY.

2 TRUSS LOADING SCHEDULE

SPECIFIED ANCHOR	ALTERNATE ANCHOR	APPLICATION	NOTES
1/2" DIA.	MASA	EXTERIOR	4
LSTDH8	HD3B, HDU2	EXTERIOR	1, 2, & 3
MPAHD	STDH10, HD3B	EXTERIOR	1, 2, & 3
STDH10	STDH14, HD5B	EXTERIOR	1, 2, & 3
HD3B	HDU2, DT12Z	INTERIOR	1, 2, & 3
HD5B	HTD5, HTT4	INTERIOR	1, 2, & 3
HDU5	HD5B, HTT5	INTERIOR	1, 2, & 3
HD7B	HDU8	INTERIOR	1, 2, & 3
LTT20B	HD5B	INTERIOR	1, 2, & 3
HTT4	STDH10	EXTERIOR	1, 2, & 3
HTT5	STDH14, (2)STDH10	EXTERIOR	1, 2, & 3

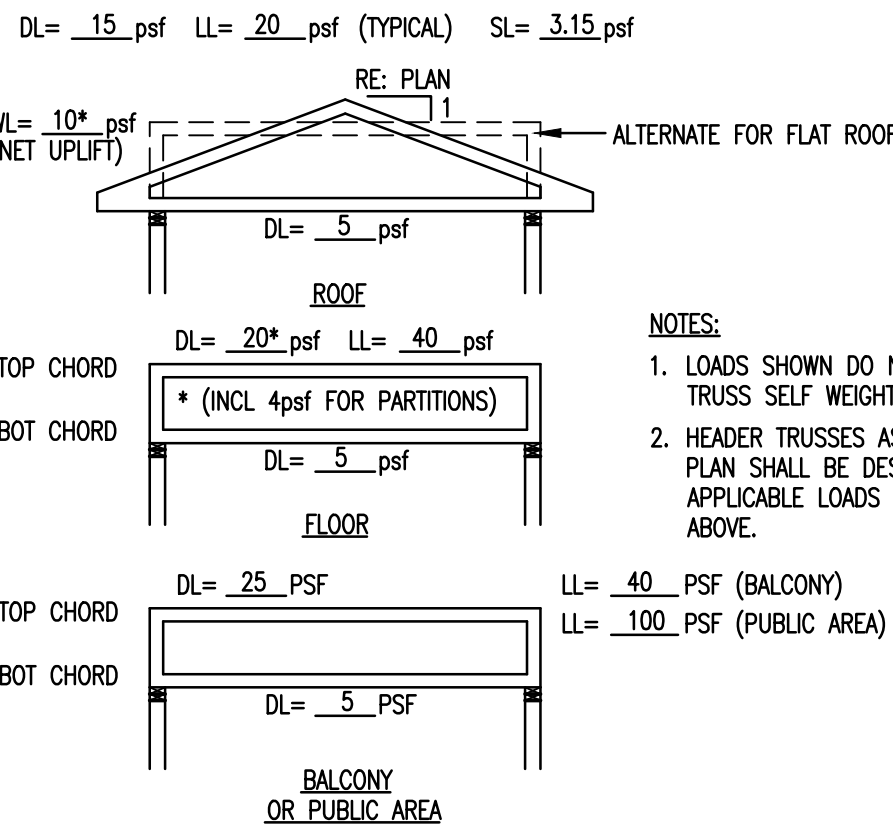
- NOTES:
- INSTALL HOLDOWN 10" MINIMUM FROM EDGE OF CONCRETE SLAB.
 - HOLDOWNS SHALL BE INSTALLED ACCORDING TO RECOMMENDATIONS BY SIMPSON STRON-TIE.
 - ALTERNATES SHOWN ABOVE ARE ACCEPTABLE SUBSTITUTES FOR SIZES SHOWN ON FIRST FLOOR BRACING PLANS.
 - REFER TO DETAIL 9/50-2.

6 ALTERNATE HOLD DOWN SCHEDULE

SHEARWALL SCHEDULE													
WALL TYPE	WALL MATERIAL				EDGE NAILING		SILL BOLTS		SOLE PLATE NAILING		REMARKS	SILL P.A.F. HILTI X-CP72 PB S23	
MARK	TYPE	THICKNESS	SIDES	SPA	SIZE	SPA	SIZE	SPA	SIZE	RE: NOTES	CAPACITY	SPA	SIZE
[1]	GYPSEATH	5/8" MIN.	1	4"	6d COOLER	48"	1/2"	16d		1, 2, 5, 7	75 PLF	40"	0.145"
[2]	GYPSEATH	5/8"	1	4"	6d COOLER	48"	1/2"	16d		1, 2, 6, 7	125 PLF	24"	0.145"
[3]	GYPSUM BOARD	5/8"	1	4"	6d COOLER	42"	1/2"	12"	16d	1, 2, 6, 7	145 PLF	20"	0.145"
[X]	GYPSUM BOARD	5/8" TWO PLY	1	4"	6d COOLER	32"	1/2"	12"	16d	1, 2, 6, 7, 10	250 PLF	12"	0.145"
[4]	GYPSUM BOARD	5/8"	1	4"	6d COOLER	36"	1/2"	10"	16d	1, 6, 7, 8, 9, 10	175 PLF	16"	0.145"
[5]	PLYWOOD/OSB	15/32"	1	4"	8d COMMON	32"	1/2"	6"	16d	1, 7, 10	380 PLF	7"	0.145"
[6]	PLYWOOD/OSB	15/32"	1	3"	10d COMMON	24"	1/2"	4"	16d	1, 7, 10	490 PLF	6"	0.145"

- NOTES:
- NAIL SPACING INDICATED IS FOR EDGE NAILING OF THE SPECIFIED WALL MATERIAL TYPICAL INTERMEDIATE STUDS (FIELD NAILING) AND BLOCKING, IF PRESENT, SHALL BE NAILED AT 4" O.C. @ GYPSEATH AND GYPSEATH BOARD, 6" O.C. @ PLYWOOD / O.S.B. SHEARWALLS.
 - WALL TYPES ARE UNBLOCKED CONSTRUCTION.
 - 11 GAGE NAILS SHALL BE 1 3/4" LONG, 7/16" DIA. DIAMOND POINT AND GALVANIZED.
 - SHEARWALL LENGTHS NOTED ON PLANS HAVE BEEN ADJUSTED TO PROVIDE CLEARANCES FOR LOCATING AND INSTALLING HOLDOWNS WHEN REQUIRED. CONTRACTOR SHALL CONSTRUCT SHEARWALL LENGTHS TO ACTUAL WALL DIMENSIONS SHOWN ON ARCHITECTURAL DRAWINGS.
 - SHEET SIZE MAY BE 2' x 8'.
 - SINKER NAILS MAY BE SUBSTITUTED PROVIDED THE SHEAR CAPACITY MATCHES COOLER NAILS.
 - ALL SHEARWALLS SHALL BE FRAMED WITH WIND POST AT EACH END. WIND POST CAN BE A 4x4 OR (2)-2x4 NAILED W/ 16d @ 12" O.C. EACH FACE.
 - WALL TYPE IS BLOCKED CONSTRUCTION.
 - SHEET SIZE TO BE 4'x8'.
 - VALUES FOR P.A.F. ARE FOR MINIMUM 2 3/4" EDGE DISTANCE (I.E. MIDDLE OF CONCRETE).

11 SHEARWALL SCHEDULE



3 BUILDING LOADS DIAGRAM

MAXIMUM SPAN LENGTH (FT)	LINTEL SIZE
0 to 4'-0"	L 3 1/2 x 3 1/2 x 5/16
4'-0" to 6'-0"	L 5 x 3 1/2 x 5/16 LLV
6'-0" to 8'-0"	L 5 x 3 1/2 x 3/8 LLV
8'-0" to 10'-0"	L 6 x 4 x 3/8 LLV
10'-0" to 12'-0"	L 7 x 4 x 3/8 LLV
> 12'-0"	COORDINATE W/ENGINEER

- NOTES:
- LINTEL ANGLE SHALL BEAR A MINIMUM OF 8" AT SUPPORTS.
 - LINTEL SHALL NOT BE SHORED DURING CONSTRUCTION.
 - LLV = LONG LEG VERTICAL.

7 LOOSE LINTEL SCHEDULE

CONTINUOUS LOAD PATH SCHEDULE									
① ROOF TO TOP PLATE		WIND SPEED 3 SEC GUST	② 2A TOP PLT. TO STUD		④ FLOOR TO FLOOR		③ STUD TO BOT. PLT.		
MODEL NO.	UPLIFT (LBS)	NOTES	M.P.H.	TYPE	SPACING	TYPE	SPACING	TYPE	SPACING
H2.5A	<415	1, 3, & 5	110 OR LESS	H2.5A	48" O.C.	CS16x36	48" O.C.	H2.5A	48" O.C.
(2) - H2.5A	415-830	3 & 5							
H2.5A	<600	3 & 6	115	H2.5A	24" O.C.	CS16x36	48" O.C.	H2.5A	24" O.C.
(2) - H2.5A	600-1200	3 & 6							
H6	<915	2, 3, & 7	120	H2.5A	24" O.C.	CS16x36	48" O.C.	H2.5A	24" O.C.
(2) - H6	915-1830	3 & 7							

ALL CONNECTIONS ARE SIMPSON STRONG-TIE CONNECTIONS.

NOTES:

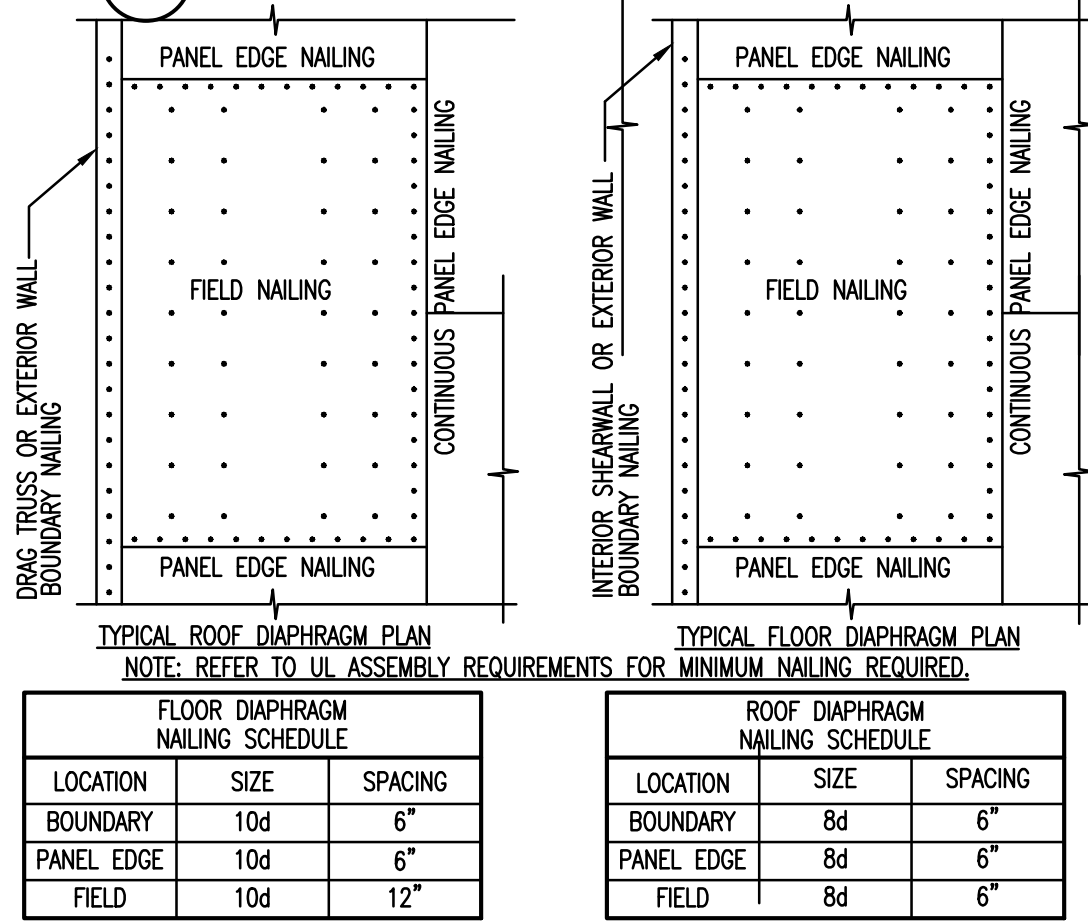
- EVERY OTHER TYPICAL ROOF TRUSS SHALL HAVE (1) - H2.5 CONNECTOR AT EACH END UNLESS TRUSS MANUFACTURER'S BEARING REACTIONS AS NOTED ON SHOP DRAWINGS ARE HIGHER THAN SPECIFIED UPLIFT.
- TYPICAL ROOF GIRDER TRUSSES SHALL HAVE (2) - H6 CONNECTOR AT EACH END UNLESS TRUSS MANUFACTURER'S REACTIONS AS NOTED ON SHOP DRAWINGS ARE HIGHER THAN SPECIFIED UPLIFT.
- SCHEDULED UPLIFT VALUES INCLUDE THE CAPACITY OF (2) - 16d ERECTION NAILS INSTALLED BY TOENAILING TRUSS BOTTOM CHORD TO BEARING PLATE.
- UPLIFT VALUES ARE IN ACCORDANCE WITH LOAD DURATION FACTORS SET FORTH BY NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION, CURRENT EDITION.
- USE (5) - 8d NAILS TO TRUSS CHORDS AND TO PLATES FOR EACH CONNECTOR SCHEDULED.
- USE (4) - 8d NAILS TO TRUSS CHORDS AND TO PLATES FOR EACH CONNECTOR SCHEDULED.
- USE (4) - 8d NAILS TO TRUSS CHORDS, (2) - 8d TO PLATES, AND (8) - 8d TO STUDS FOR EACH CONNECTOR SCHEDULED.

4 CONTINUOUS LOAD PATH

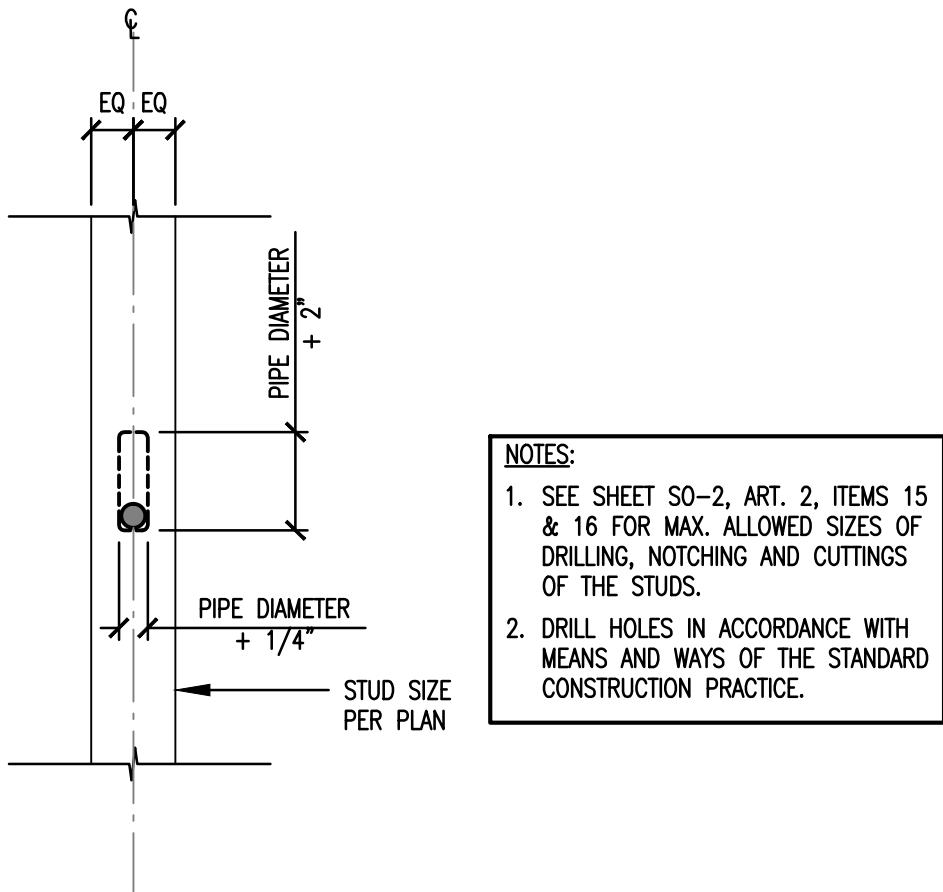
LUMBER		
POST SIZE LOCATION	SIMPSON EMBEDDED POST BASE	SIMPSON BOLTED POST BASE
4x4	CB44	ABU44
6x6	CB66	ABU66
8x8	CB88	ABU88

1.8E PARALLAM PSL		
COLUMN PRODUCT	COLUMN SIZE	CONNECTOR
1.8E PARALLAM PSL	3 1/2"x3 1/2"	LCB44
	3 1/2"x 5 1/4"	CB44
	3 1/2"x 7"	CB7 1/8-4
	5 1/4"x 5 1/4"	CB66
	5 1/4"x 7"	CB6-7
	7"x 7"	CB7 1/8-7

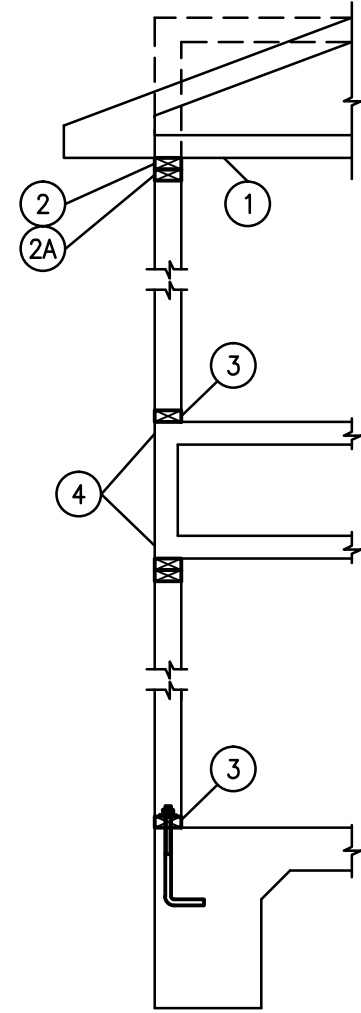
8 POST BASE SCHEDULE



12 DIAPHRAGM NAILING DIAGRAM



RECOMMENDATION FOR PLUMBING HOLE IN WOOD STUDS

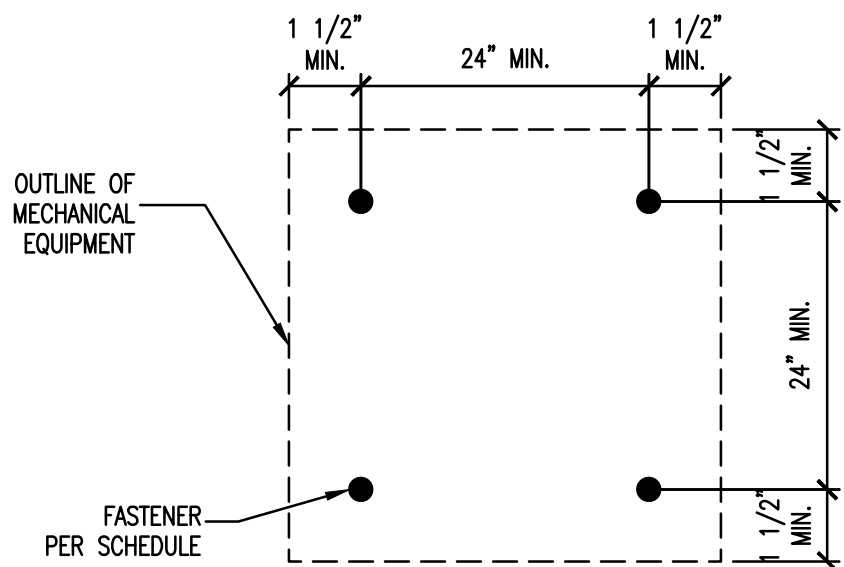


ATTACHMENT ANCHOR TYPE	EMBEDMENT	LOCATION AND SPACING			NOTES
		EXTERIOR WALLS	INTERIOR WALLS	NON-LOAD BEARING WALLS	
1/2" DIA. ANCHOR BOLTS	7"	48" O.C.	72" O.C.	N/A	2, 3 & 4
1/2" DIA. EXPANSION ANCHORS	2 1/4"	N/A	72" O.C.	N/A	1 & 3
0.117" DIA. POWDER ACTUATED FASTENERS	1 1/2"	12"	24" O.C.	48" O.C.	3 & 5
0.099" DIA. POWDER ACTUATED FASTENERS	1"	N/A	12" O.C.	12" O.C.	3

NOTES:

- EXPANSION ANCHORS SHALL NOT BE ALLOWED WITHIN 10 INCHES OF SLAB EDGE.
- REFER TO SHEARWALL SCHEDULE FOR ANCHORAGE REQUIREMENTS.
- RE: PLAN FOR ADDITIONAL NOTES.
- ALL HARDWARE IN CONTACT WITH ACQ TREATED LUMBER CLASS G185 MUST BE SIMPSON ZMAX PRODUCTS THAT MEET ASTM A653.
- MIN. EDGE DISTANCE FOR P.A.F. AT EXT. BEARING WALLS SHALL BE 2 3/4".

9 BEARING WALL ANCHOR SCHEDULE



WIND SPEED (MPH)	90	100	110	120	130
FASTENER EMBEDMENT DEPTH	1"	1"	1"	1 1/2"	1 1/2"

NOTES:

- APPLIES TO GROUND LOCATED MECHANICAL EQUIPMENT MAXIMUM DIMENSIONS OF 3'x3'x3'.
<

TENDON ELONGATION SCHEDULE					
LENGTH (FEET)	ELONGATION (INCHES)	LENGTH (FEET)	ELONGATION (INCHES)	LENGTH (FEET)	ELONGATION (INCHES)
10	0.8	64	5.1	118	9.3
12	0.9	66	5.2	120	9.5
14	1.1	68	5.4	122	9.6
16	1.3	70	5.5	124	9.8
18	1.4	72	5.7	126	10.0
20	1.6	74	5.8	128	10.1
22	1.7	76	6.0	130	10.3
24	1.9	78	6.2	132	10.4
26	2.1	80	6.3	134	10.6
28	2.2	82	6.5	136	10.7
30	2.4	84	6.6	138	10.9
32	2.5	86	6.8	140	11.1
34	2.7	88	7.0	142	11.2
36	2.8	90	7.1	144	11.4
38	3.0	92	7.3	146	11.5
40	3.2	94	7.4	148	11.7
42	3.3	96	7.6	150	11.9
44	3.5	98	7.7	152	12.0
46	3.6	100	7.9	154	12.2
48	3.8	102	8.1	156	12.3
50	4.0	104	8.2	158	12.5
52	4.1	106	8.4	160	12.6
54	4.3	108	8.5	162	12.8
56	4.4	110	8.7	164	13.0
58	4.6	112	8.8	166	13.1
60	4.7	114	9.0	168	13.3
62	4.9	116	9.2	170	13.4

NOTE:
1. THE TESTING LAB SHALL OBTAIN TENDON SHOP DRAWINGS FROM THE SUPPLIER TO DETERMINE THE INDIVIDUAL ELONGATIONS OF THE CABLES.

PLAN LEGEND

1. BEAM WIDTHS / DEPTHS SHALL BE AS SHOWN IN GRADE BEAM SCHEDULE, U.N.O.

2. PROVIDE 4-#3 x 3'-0" AT OPENINGS LARGER THAN 10".

3.

TENDON NUMBER

SLAB OR BEAM TENDON DESIGNATION

TENDON LIVE END

SLAB EDGE AND FACE OF GRADE BEAM

4. SYMBOLS:

SLAB TENDON DEAD END

SLAB TENDON LIVE END

BEAM TENDON DEAD END

BEAM TENDON LIVE END

SHEARWALL

DIAGONAL REBAR
2-#4x6'-0" UNLESS NOTED OTHERWISE.

FLOOR FRAMING NOTES

1. TRUSS SPACING SHALL BE 24" ON CENTER UNLESS NOTED OTHERWISE.

2. EXTERIOR BEARING WALLS AND / OR SHEARWALLS LOCATED PARALLEL TO FLOOR TRUSS FRAMING SHALL HAVE CONTINUOUS BEARING MEMBERS ABOVE PLATE LINE TO SUPPORT WALLS ABOVE.

3. ARCHITECTURAL BACKGROUND SHOWN IS LAYOUT OF FLOOR BELOW. SHOWN THUS:

4. SHEARWALLS AS NOTED ON PLAN SHALL BE CONSTRUCTED FROM FRAMING LEVEL BELOW TO FRAMING LEVEL SHOWN.

5. HOLDINGS AND / OR STRAPS SHOWN SHALL BE MANUFACTURED BY SIMPSON STRONG TIE OR APPROVED EQUAL.

6. BEARING WALLS ARE INDICATED THUS:

7. VERIFY ALL FLOOR TOP OF PLATE ELEVATIONS WITH ARCHITECTURAL PLANS AND WALL SECTIONS.

8. NON LOAD-BEARING PARTITIONS 10'-0" OR LONGER IN LENGTH SHALL HAVE CONTINUOUS BEARING MEMBERS SUPPORT FROM WALLS OR FLOOR TRUSSES BELOW.

9. THE TRUSS LAYOUT SHOWN ON THIS DRAWING REPRESENTS DIRECTION OF TRUSS SPANS ONLY. THIS DRAWING SHALL NOT BE USED FOR PLACEMENT OF TRUSSES. REFER TO APPROVED TRUSS MANUFACTURER'S DRAWINGS FOR PLACEMENT, DIMENSIONS, BRACING AND CONNECTIONS.

10. QUANTITIES SHOWN ARE FOR INFORMATIONAL PURPOSES AND SHOULD BE CONFIRMED WITH THE ARCHITECTURAL DRAWINGS.

11. REFER TO GENERAL NOTES AND DIAGRAM FOR NAILING, STUD, HEADER, SHEARWALL BRACING, LOOSE INTEL SCHEDULE, AND OTHER INFORMATION NOT SHOWN ON PLAN.

12. VERIFY ALL FRAMING DIMENSIONS AND BACKGROUNDS WITH THE ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES TO THE STRUCTURAL ENGINEER.

13. PROVIDE MINIMUM DOUBLE 2x STUD UNDERNEATH EACH GIRDER TRUSS.

FLOOR FRAMING LEGEND

- HEADER

- DROP BEAM

- FLUSH BEAM

- FLOOR/BALCONY/CORRIDOR TRUSS

- GIRDER TRUSS

- BALCONY TRUSS

- CORRIDOR TRUSS

- CONTINUOUS BEARING TRUSS

- FLOOR TRUSS

- GIRDER TRUSS

- ROOF TRUSS

- STRAP, FOR SIZE RE: PLAN

- PARALLEL STRAND LUMBER

- GLULAM BEAM

- CONTINUOUS

- NON-LOAD BEARING WALL ABOVE DOUBLE UNLESS NOTED OTHERWISE

- BRIDGING

- UNDER WALL ABOVE

WOOD BEAM SCHEDULE		
PLAN MARK	BEAM TO USE *	SIMPSON HANGER
226	(2) 2 x 6	HU26-2
228	(2) 2 x 8	HU28-2
2210	(2) 2 x 10	HU210-2
2212	(2) 2 x 12	HU212-2
326	(3) 2 x 6	HU26-3
328	(3) 2 x 8	HU28-3
3210	(3) 2 x 10	HU210-3
3212	(3) 2 x 12	HU212-3
4212	(4) 2 x 12	HHUS210-4

PSL BEAM SCHEDULE		
B408	3 1/2" x 7 1/4"	HGUS410
B409	3 1/2" x 9 1/4"	HGUS412
B411	3 1/2" x 11 1/4"	HGUS412
B412	3 1/2" x 11 7/8"	HGUS412
B414	3 1/2" x 14"	HGUS414
B416	3 1/2" x 16"	HGUS414
B418	3 1/2" x 18"	HGUS414
B609	5 1/4" X 9 1/4"	HGUS5.50/10
B611	5 1/4" x 11 1/4"	HGUS5.50/12
B612	5 1/4" x 11 7/8"	HGUS5.50/12
B614	5 1/4" x 14"	HGUS5.50/14
B616	5 1/4" x 16"	HGUS5.50/14
B618	5 1/4" x 18"	HGUS5.50/14
B711	7" x 11 1/4"	HGUS7.25/12
B712	7" x 11 7/8"	HGUS7.25/12
B714	7" x 14"	HGUS7.25/14
B716	7" x 16"	HGUS7.25/14
B718	7" x 18"	HGUS7.25/14

* NOTE:
REFER TO GENERAL NOTES FOR MEMBER PROPERTIES.

LOAD BEARING HEADER SCHEDULE		
(#2 SYP OR #2 D. FIR)		
CLEAR SPAN	SIZE (UNLESS NOTED)	REMARKS
≤2'-0"	2-2x6	1 , 2
2'-1 1/2 4'-0"	2-2x8	1 , 2
4'-1 1/2 5'-0"	2-2x10	1 , 2
5'-1 1/2 6'-0"	2-2x12	1 , 2
6'-1 1/2 8'-0"	2-2x12	1 , 2
8'-1 1/2 10'-0"	2-2x12	1 , 2

NOTES:

1. MULTIPLE LUMBER BEAMS SHALL BE CONSTRUCTED WITH A 1/2" THICK OSB/PLYWOOD SPACER BETWEEN MEMBERS. MEMBERS SHALL BE NAILED TOGETHER USING 16d NAILS @ 12" O.C. TOP AND BOTTOM (STAGGERED).

2. HEADERS LOCATED IN DESIGNATED BEARING WALLS AND IN WALLS SUBJECT TO EXTERIOR WIND LOAD ARE CONSIDERED LOAD BEARING.

NON LOAD BEARING HEADER SCHEDULE		
(#2 SYP OR #2 D. FIR)		
CLEAR SPAN	SIZE (UNLESS NOTED)	REMARKS
≤ 4'-0"	2-2x6	1
4'-1 1/2 6'-0"	2-2x8	1
6'-1 1/2 8'-0"	2-2x10	1
8'-1 1/2 10'-0"	2-2x12	1

NOTES:

1. MULTIPLE LUMBER BEAMS SHALL BE CONSTRUCTED WITH A 1/2" THICK OSB/PLYWOOD SPACER BETWEEN MEMBERS. MEMBERS SHALL BE NAILED TOGETHER USING 16D NAILS @ 12" O.C. TOP AND BOTTOM (STAGGERED).

ROOF FRAMING NOTES

1. TRUSS SPACING SHALL BE 24" ON CENTER UNLESS NOTED OTHERWISE.

2. ARCHITECTURAL BACKGROUND SHOWN IS LAYOUT OF FLOOR BELOW.

3. BEARING WALLS ARE INDICATED THUS:

4. TOP OF PLATES AT ROOF MAY VARY FROM ELEVATIONS SHOWN ON PLANS. REFER TO ARCHITECTURAL DRAWINGS FOR CONDITIONS NOTED DIFFERENTLY ON THE STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCIES TO STRUCTURAL ENGINEER.

5. VERIFY ALL FRAMING DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES TO STRUCTURAL ENGINEER.

6. AREAS SHADED ON PLAN REPRESENT OVERLAY CONDITIONS. FRAME WITH 2x6 AT 24" ON CENTER MAXIMUM WITH 2x8 RIDGE.

7. ROOF TRUSSES SHALL BE ANCHORED AT BEARING SUPPORTS WITH HURRICANE TIES AT 24" ON CENTER MAXIMUM USING SIMPSON H2.5 TIES.

8. CONTRACTOR SHALL PROVIDE ERECTION BRACING FOR ROOF FRAMING INSTALLATION IN ACCORDANCE WITH TRUSS MANUFACTURER'S RECOMMENDATIONS UNLESS NOTED OTHERWISE.

9. REFER TO ARCHITECTURAL DRAWINGS FOR ALL ROOF SLOPES.

10. THE TRUSS LAYOUT SHOWN ON THIS DRAWING REPRESENTS DIRECTION OF TRUSS SPANS ONLY. THIS DRAWING SHALL NOT BE USED FOR PLACEMENT OF TRUSSES. REFER TO APPROVED TRUSS MANUFACTURER'S DRAWINGS FOR PLACEMENT, DIMENSIONS, BRACING AND CONNECTIONS.

11. PROVIDE MINIMUM DOUBLE 2x STUDS UNDERNEATH EACH GIRDER TRUSS.

ROOF FRAMING LEGEND

- HEADER

- DROP BEAM

- FLUSH BEAM

- ROOF TRUSS

- WALL TRUSS

- GIRDER TRUSS

- HANGER BY SIMPSON

- OVERLAID FRAMING AREA

- STRONGBACK OR DIAGONAL BRACING

1 FOUNDATION SCHEDULE

SHEARWALL NOTES

1. SHEARWALL WITH OPENINGS MAY REQUIRE HARDWARE AT CORNERS OF OPENINGS. RE: SHEARWALL DETAIL SHEET FOR DETAILS.

2. REFER TO SHEET SD3-1 FOR SHEARWALL DETAILS.

SHEARWALL LEGEND

SHEARWALL HOLDOWN OR ANCHOR SHOWN ON PLAN

SHEARWALL

SHEARWALL TYPE CALLED OUT THIS SIDE (PLAN REFERENCE)

SHEARWALL TYPE OPPOSITE SIDE (PLAN REFERENCE)

SHEARWALL LENGTH IN FT.

SHEARWALL HOLDOWN OR ANCHOR SHOWN ON PLAN

SHEARWALL

SHEARWALL TYPE CALLED OUT THIS SIDE (PLAN REFERENCE)

SHEARWALL TYPE OPPOSITE SIDE (PLAN REFERENCE)

SHEARWALL LENGTH IN FT.

2 FLOOR FRAMING NOTES

ONE STORY SCHEDULE		
(STUD GRADE SYP OR STUD GRADE D. FIR)		
INTER. LOAD BEARING TRIMMER/KING POST SCHEDULE		
CLEAR SPAN	STUDS	1ST FLOOR
≤2'-0"	TRIMMER KING POST	1 1
2'-1" ≤ 4'-0"	TRIMMER KING POST	1 1
4'-1" ≤ 5'-0"	TRIMMER KING POST	1 1
5'-1" ≤ 6'-0"	TRIMMER KING POST	1 1
6'-1" ≤ 8'-0"	TRIMMER KING POST	2 1
8'-1" ≤ 10'-0"	TRIMMER KING POST	2 1

ONE STORY SCHEDULE		
(STUD GRADE SYP OR STUD GRADE D. FIR)		
LOAD BEARING TRIMMER/KING POST SCHEDULE		
CLEAR SPAN	STUDS	1ST FLOOR
≤2'-0"	TRIMMER KING POST	1 1
2'-1" ≤ 4'-0"	TRIMMER KING POST	1 1
4'-1" ≤ 5'-0"	TRIMMER KING POST	1 1
5'-1" ≤ 6'-0"	TRIMMER KING POST	1 2
6'-1" ≤ 8'-0"	TRIMMER KING POST	2 2
8'-1" ≤ 10'-0"	TRIMMER KING POST	2 3

3 BEAM SCHEDULE

TWO STORY SCHEDULE			
(#2 SYP OR #2 D. FIR)			
EXT. LOAD BEARING TRIMMER/KING POST SCHEDULE (FLOOR TRUSSES PARALLEL TO WALLS)			
CLEAR SPAN	STUDS	1ST FLOOR	2ND FLOOR
≤2'-0"	TRIMMER KING POST	1 1	1 1
2'-1 1/2 4'-0"	TRIMMER KING POST	1 2	1 2
4'-1 1/2 5'-0"	TRIMMER KING POST	1 2	1 2
5'-1 1/2 6'-0"	TRIMMER KING POST	2 2	1 2
6'-1 1/2 8'-0"	TRIMMER KING POST	2 3	2 3
8'-1 1/2 10'-0"	TRIMMER KING POST	2 3	2 3

TWO STORY SCHEDULE			
(#2 SYP OR #2 D. FIR)			
INTERIOR LOAD BEARING TRIMMER/KING POST SCHEDULE			
CLEAR SPAN	STUDS	1ST FLOOR	2ND FLOOR
≤2'-0"	TRIMMER KING POST	1 1	1 1
2'-1" ≤ 4'-0"	TRIMMER KING POST	1 1	1 1
4'-1" ≤ 5'-0"	TRIMMER KING POST	1 1	1 1
5'-1" ≤ 6'-0"	TRIMMER KING POST	2 1	1 1
6'-1" ≤ 8'-0"	TRIMMER KING POST	2 1	2 1
8'-1" ≤ 10'-0"	TRIMMER KING POST	3 1	2 1

TWO STORY SCHEDULE			
(#2 SYP OR #2 D. FIR)			
EXT. LOAD BEARING TRIMMER/KING POST SCHEDULE (ROOF/TRUSSES @ SAME DIRECTION-WORST CASE)			
CLEAR SPAN	STUDS	1ST FLOOR	2ND FLOOR
≤2'-0"	TRIMMER KING POST	1 1	1 1
2'-1 1/2 4'-0"	TRIMMER KING POST	1 2	1 2
4'-1" ≤ 5'-0"	TRIMMER KING POST	1 2	1 2
5'-1" ≤ 6'-0"	TRIMMER KING POST	2 2	1 2
6'-1" ≤ 8'-0"	TRIMMER KING POST	2 3	2 3
8'-1" ≤ 10'-0"	TRIMMER KING POST	3 3	2 3

4 LOAD BEARING AND NON LOAD BEARING HEADER SCHEDULE

THREE STORY SCHEDULE				
(#2 SYP)				
EXT. LOAD BEARING TRIMMER/KING POST SCHEDULE (ROOF/FLOOR TRUSSES @ SAME DIRECTION-WORST CASE)				
CLEAR SPAN	STUDS	1ST FLOOR	2ND FLOOR	3RD FLOOR
≤2'-0"	TRIMMER KING POST	1 1	1 1	1 1
2'-1 1/2 4'-0"	TRIMMER KING POST	2 2	1 2	1 2
4'-1 1/2 5'-0"	TRIMMER KING POST	2 2	1 2	1 2
5'-1 1/2 6'-0"	TRIMMER KING POST	2 2	2 2	1 2
6'-1" ≤ 8'-0"	TRIMMER KING POST	3 3	2 3	2 3
8'-1 1/2 10'-0"	TRIMMER KING POST	4 3	3 3	2 3

THREE STORY SCHEDULE				
(#2 SYP OR BETTER) (U.N.O. ON PLANS)				
INTERIOR LOAD BEARING TRIMMER/KING POST SCHEDULE				
CLEAR SPAN	STUDS	1ST FLOOR	2ND FLOOR	3RD FLOOR
≤ 2'-0"	TRIMMER KING POST	1 1	1 1	1 1
2'-1" ≤ 4'-0"	TRIMMER KING POST	2 1	1 1	1 1
4'-1" ≤ 5'-0"	TRIMMER KING POST	2 1	1 1	1 1
5'-1" ≤ 6'-0"	TRIMMER KING POST	2 1	2 1	1 1
6'-1" ≤ 8'-0"	TRIMMER KING POST	3 1	2 1	2 1
8'-1" ≤ 10'-0"	TRIMMER KING POST	4 1	3 1	2 1

THREE STORY SCHEDULE				
(#2 SYP OR BETTER) (U.N.O. ON PLANS)				
EXT. LOAD BEARING TRIMMER/KING POST SCHEDULE (FLOOR TRUSSES PARALLEL TO WALLS)				
CLEAR SPAN	STUDS	1ST FLOOR	2ND FLOOR	3RD FLOOR
≤ 2'-0"	TRIMMER KING POST	1 1	1 1	1 1
2'-1" ≤ 4'-0"	TRIMMER KING POST	2 1	2 1	1 1
4'-1" ≤ 5'-0"	TRIMMER KING POST	2 1	2 1	1 1
5'-1" ≤ 6'-0"	TRIMMER KING POST	2 2	2 2	1 2
6'-1" ≤ 8'-0"	TRIMMER KING POST	3 2	2 2	2 2
8'-1" ≤ 10'-0"	TRIMMER KING POST	4 3	3 3	2 3

5 ROOF FRAMING NOTES

STUD PACK SCHEDULE (S)'		PSL COULMN SCHEDULE (C)'	
PLAN MARK	SIZE	PLAN MARK	SIZE
S24	(2) 2x4 STUDS	C33	3-1/2"x3-1/2" PSL COL.
S34	(3) 2x4 STUDS	C35	3-1/2"x5-1/4" PSL COL.
S44	(4) 2x4 STUDS	C37	3-1/2"x7" PSL COL.
S54	(5) 2x4 STUDS	C39	3-1/2"x9-1/4" PSL COL.
S64	(6) 2x4 STUDS	C311	3-1/2"x11-1/4" PSL COL.
S26	(2) 2x6 STUDS	C314	3-1/2"x14" PSL COL.
S36	(3) 2x6 STUDS	C55	5-1/4"x5-1/4" PSL COL.
S46	(4) 2x6 STUDS	C57	5-1/4"x7" PSL COL.
S56	(5) 2x6 STUDS	C59	5-1/4"x9-1/4" PSL COL.
S66	(6) 2x6 STUDS	C511	5-1/4"x11-1/4" PSL COL.
S28	(2) 2x8 STUDS	C514	5-1/4"x14" PSL COL.
S38	(3) 2x8 STUDS	C77	7"x7" PSL COL.
S48	(4) 2x8 STUDS	C79	7"x9-1/4" PSL COL.
S58	(5) 2x8 STUDS	C711	7"x11-1/4" PSL COL.
S68	(6) 2x8 STUDS	C714	7"x14" PSL COL.

POST SCHEDULE (P)'		STEEL COLUMN SCHEDULE (SC) ²	
PLAN MARK	SIZE	PLAN MARK	SIZE
P44	4"x4" #2 POST		
P46	4"x6" #2 POST		
P48	4"x8" #2 POST		
P66	6"x6" #2 POST		
P68	6"x8" #2 POST		
P88	8"x8" #2 POST		

6 SHEARWALL SCHEDULE

7 STORY SCHEDULE

8 COLUMN & STUD PACKS SCHEDULE

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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

Sheet Title

Date

Description

Rev.

Drawn By: HT
Checked By: DMH/ZA
Drawing Scale: As Noted
Project No. 136-091

ISSUED FOR: DATE:

☐ SD 30%
☐ Coordination
☐ CD 95%
☐ CD 100%
☐ Pricing
☐ Bidding
☐ Permit
☐ Construction

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TABLE 2304.9.1
FASTENING SCHEDULE (IBC 2015)

CONNECTION	FASTENING ^{a, m}	LOCATION
1. Joist to sill or girder	3 - 8d common (2 ½" x 0.131") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	toenail
2. Bridging to joist	2 - 8d common (2 ½" x 0.131") 2 - 3" x 0.131" nails 2 - 3" 14 gage staples	toenail each end
3. 1" x 6" subfloor or less to each joist	2 - 8d common (2 ½" x 0.131")	face nail
4. Wider than 1" x 6" subfloor to each joist	3 - 8d common (2 ½" x 0.131")	face nail
5. 2" subfloor to joist or girder	2 - 16d common (3 ½" x 0.162")	blind and face nail
6. Sole plate to joist or blocking	16d (3 ½" x 0.135") at 16" o.c. 3" x 0.131" nails at 8" o.c. 3" 14 gage staples at 12" o.c.	typical face nail
Sole plate to joist or blocking at braced wall panel	3 - 16d (3 ½" x 0.135") at 16" o.c. 4 - 3" x 0.131" nails at 16" o.c. 4 - 3" 14 gage staples at 16" o.c.	braced wall panels
7. Top plate to stud	2 - 16d common (3 ½" x 0.162") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	end nail
8. Stud to sole plate	4 - 8d common (2 ½" x 0.131") 4 - 3" x 0.131" nails 3 - 3" 14 gage staples	toenail
	2 - 16d common (3 ½" x 0.162") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	end nails
9. Double studs	16d (3 ½" x 0.135") at 24" o.c. 3" x 0.131" nail at 8" o.c. 3" 14 gage staple at 8" o.c.	face nail
10. Double top plates	16d (3 ½" x 0.135") at 16" o.c. 3" x 0.131" nail at 12" o.c. 3" 14 gage staple at 12" o.c.	typical face nail
	8 - 16d common (3 ½" x 0.162") 12 - 3" x 0.131" nails 12 - 3" 14 gage staples	lap splice
11. Blocking between joists or rafters to top plate	3 - 8d common (2 ½" x 0.131") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	toenail
12. Rim joist to top plate	8d (2 ½" x 0.131") at 6" o.c. 3" x 0.131" nail at 6" o.c. 3" 14 gage staple at 6" o.c.	toenail
13. Top plates, laps and intersections	2 - 16d common (3 ½" x 0.162") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	face nail
14. Continuous header, two pieces	16d common (3 ½" x 0.162")	16" o.c. along edge
15. Ceiling joints to plate	3 - 8d common (2 1/2" x 0.131") 5 - 3" x 0.131" nails 5 - 3" 14 gage staples	toenail
16. Continuous header to stud	4 - 8d common (2 ½" x 0.131")	toenail
17. Ceiling Joists, laps over partitions (see Section 2308.10.4.1, Table 2308.10.4.1)	3 - 16d common (3 ½" x 0.162") minimum, Table 2308.10.4.1 4 - 3" x 0.131" nails 4 - 3" 14 gage staples	face nail
18. Ceiling Joists to parallel rafters (see Section 2308.10.4.1, Table 2308.10.4.1)	3 - 16d common (3 ½" x 0.162") minimum, Table 2308.10.4.1 4 - 3" x 0.131" nails 4 - 3" 14 gage staples	face nail
19. Rafter to plate (see Section 2308.10.1, Table 2308.10.1)	3 - 8d common (2 ½" x 0.131") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	toenail
20. 1" diagonal brace to each stud and plate	2 - 8d common (2 ½" x 0.131") 2 - 3" x 0.131" nails 3 - 3" 14 gage staples	face nail
21. 1" x 8" sheathing to each bearing	3 - 8d common (2 ½" x 0.131")	face nail
22. Wider than 1" x 8" sheathing to each bearing	3 - 8d common (2 ½" x 0.131")	face nail
23. Built-up corner studs	16d common (3 ½" x 0.162") 3" x 0.131" nails 3" 14 gage staples	24" o.c. 16" o.c. 16" o.c.
24. Built-up girder and beams	20d common (4" x 0.192") 32" o.c. 3" x 0.131" nails at 24" o.c. 3" 14 gage staple at 24" o.c.	face nail at top and bottom staggered on opposite sides
	2 - 20d common (4" x 0.192") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	face nail at ends and at each splice
	16d common (3 ½" x 0.162")	at each bearing
26. Collar tie to rafter	3 - 10d common (3" x 0.148") 4 - 3" x 0.131" nails 4 - 3" 14 gage staples	face nail
27. Jack rafter to hip	3 - 10d common (3" x 0.148") 4 - 3" x 0.131" nails 4 - 3" 14 gage staples	toenail
	2 - 16d common (3 ½" x 0.162") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	face nail
	2 - 16d common (3 ½" x 0.162") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	toenail
28. Roof rafter to 2-by ridge beam	2 - 16d common (3 ½" x 0.162") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	face nail
	2 - 16d common (3 ½" x 0.162") 3 - 3" x 0.131" nails 3 - 3" 14 gage staples	face nail

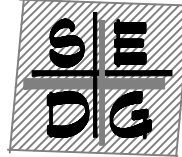
TABLE 2304.9.1-continued
FASTENING SCHEDULE (IBC 2015)

CONNECTION	FASTENING ^{a, m}	LOCATION
29. Joist to band joist	3 - 16d common (3 ½" x 0.162") 4 - 3" x 0.131" nails 4 - 3" 14 gage staples	face nail
30. Ledger strip	3 - 16d common (3 ½" x 0.162") 4 - 3" x 0.131" nails 4 - 3" 14 gage staples	face nail at each joist
31. Wood structural panels and particleboard ^b Subfloor, roof and wall sheathing (to framing)	1/2" and less 6d ⁿ¹ 2 ¾" x 0.113" nail ⁿ 1 ¾" 16 gage ⁿ 8d ⁿ or 6d ⁿ 19/32" to 3/4" 2 ¾" x 0.113" nail ^p 2" 16 gage ^p 7/8" to 1" 8d ⁿ 1 ½" to 1 ¾" 10d ⁿ or 8d ⁿ Single floor (combination subfloor-underlay-ment to framing) 3/4" and less 6d ⁿ 7/8" to 1" 8d ⁿ 1 ½" to 1 ¾" 10d ⁿ or 8d ⁿ	face nail
32. Panel siding (to framing)	1/2" or less 6d ^f 5/8" 8d ^f	
33. Fiberboard sheathing ^g	1/2" No. 11 gage roofing nail ^h 6d common nail (2" x 0.113") No. 16 gage staple ⁱ No. 11 gage roofing nail ^h 25/32" 8d common nail (2 ½" x 0.131") No. 16 gage staple ⁱ	
34. Interior paneling	1/4" 4d ^j 3/8" 6d ^k	

For SI: 1 inch = 25.4 mm.

- a. Common or box nails are permitted to be used except where otherwise stated.
- b. Nails spaced at 6 inches on center at edges, 12 inches at intermediate supports except 6 inches at supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shear walls, refer to Section 2305. Nails for wall sheathing are permitted to be common, box or casing.
- c. Common or deformed shank (6d - 2" x 0.113"; 8d - 2 ½" x 0.131"; 10d - 3" x 0.148")
- d. Common (6d - 2" x 0.113; 8d - 2 ½" x 0.131"; 10d - 3" x 0.148")
- e. Deformed shank (6d - 2" x 0.113"; 8d - 2 ½" x 0.131"; 10d - 3" x 0.148")
- f. Corrosion-resistant siding (6d - 1 ¾" x 0.106"; 8d - 2 ¾" x 0.128") or casing (6d - 2" x 0.099"; 8d - 2 ½" x 0.113") nail.
- g. Fasteners spaced 3 inches on center at exterior edges and 6 inches on center at intermediate supports. when used as structural sheathing. Spacing shall be 6 inches on center on the edge and 12 inches on center at intermediate supports for nonstructural applications.
- h. Corrosion-resistant roofing nails with 7/16-inch-diameter head and 1 ½-inch length for 1/2-inch sheathing and 1 ¾-inch length for 25/32-inch sheathing.
- i. Corrosion-resistant staples with nominal 7/16-inch crown and 1-inch crown and 1 ¼-inch length for 1/2-inch sheathing and 1 ½-inch length for 25/32-inch sheathing. Panel supports at 16 inches (20 inches if strength axis in the long direction of the panel, unless otherwise marked).
- j. Casing (1 ½" x 0.080") or finish (1 ½" x 0.072") nails spaced 6 inches on panel edges, 12 inches at intermediate supports.
- k. Panel supports at 24 inches, Casing or finish nails spaced 6 inches on panel edges, 12 inches at intermediate supports.
- l. For roof sheathing applications, 8d nails (2 ½" x 0.113") are the minimum required for wood structural panels.
- m. Staples shall have a minimum crown width of 7/16 inch.
- n. For roof sheathing applications, fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.
- o. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports for subfloor and wall sheathing and 3 inches on center at edges, 6 inches at intermediate supports for roof sheathing.
- p. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.

1 NAILING SCHEDULE AND NOTES (IBC 2015)



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AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

SCHEDULES

Sheet Title

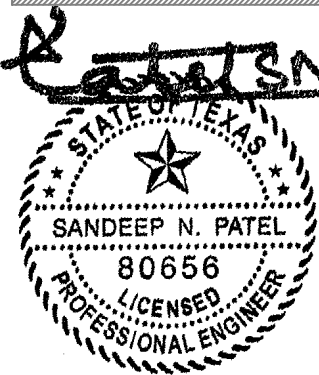
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☐ SD 30%
☐ Coordination
☐ CD 95%
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SHEET NO.

S0-6

SCHEDULES

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STATEMENT OF SPECIAL INSPECTIONS (2015 IBC)

Special Inspections may be required by local code or building official. Below is an excerpt from the IBC showing typical inspections. Actual required special inspections and their frequency should be confirmed prior to construction. This office takes no responsibility for inspections not conducted by a representative of this firm.

Table 1.19.1 – Level A Quality Assurance

MINIMUM TESTS	
None	
MINIMUM INSPECTION	
Verify compliance with the approved submittals	

Table 1.19.2 – Level B Quality Assurance

MINIMUM TESTS				
Verification of Slump flow and Visual Stability Index (VSI) as delivered to the project site in accordance with Specification Article 1.5 B.1.b.3 for self-consolidating grout.				
Verification of f'_m and f'_{wc} in accordance with Specification Article 1.4 B prior to construction, except where specifically exempted by this code.				
MINIMUM INSPECTION				
Inspection Task	Frequency ^(a)		Reference for Criteria	
	Continuous	Periodic	TMS 402 ACI 530/ ASCE 5	TMS 602/ ACI 530.1/ ASCE 6
1. Verify compliance with the approved submittals		X		Art. 1.5
2. As masonry construction begins, verify that the following are in compliance:				
a. Proportions of site-prepared mortar		X		Art. 2.1 2.6 A
b. Construction of mortar joints		X		Art. 3.3 B
c. Grade and size of prestressing tendons and anchorages		X		Art. 2.4 B, 2.4 H
d. Location of reinforcement, connectors, and prestressing tendons and anchorages		X		Art. 3.4, 3.6 A
e. Prestressing technique.		X		Art. 3.6 B
f. Properties of thin-bed mortar for AAC masonry	X ^(b)	X ^(b)		Art. 2.1 C
3. Prior to grouting, verified that the following are in compliance:				
a. Grout space		X		Art. 3.2 D 3.2 F
b. Grade, type and size of reinforcement and anchor bolts, and prestressing tendons and anchorages.		X	Sec. 1.16	Art. 2.4, 3.4
c. Placement of reinforcement, connectors, and prestressing tendons and anchorages		X	Sec. 1.16	Art. 3.2 E, 3.4, 3.6 A
d. Proportions of site-prepared grout and prestressing grout for bonded tendons		X		Art. 2.6 B 2.4 G.1.b
e. Construction of mortar joints		X		Art. 3.3 B
4. Verify during construction:				
a. Size and location of structural elements		X		Art. 3.3 F
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction		X	Sec. 1.16.4.3, 1.17.1	
c. Welding of reinforcement	X		Sec. 2.1.8.7.2, 3.3.3.4 (c) 8.3.3.4 (b)	
d. Preparation, construction and protection of masonry during cold weather (temperature below 40°F) (4.4°C) or hot weather (temperature above 90°F) (32.2°C)		X		Art. 1.8 C, 1.8 D
e. Application and measurement of prestressing force	X			Art. 3.6 B
f. Placement of grout and prestressing grout for bonded tendons is in compliance	X			Art. 3.5, 3.6 C
g. Placement of AAC masonry units and construction of thin-bed mortar joints	X ^(b)	X ^(b)		Art. 3.3 B.8
5. Observe preparation of grout specimens, mortar specimens, and/or prisms	X			Art. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4

(a) Frequency refers to the frequency of inspection, which may be continuous during the task listed or periodically during the listed task, as defined in the table.

(b) Required for the first 5000 square feet (465 square meters) of AAC masonry.
(c) Required after the first 5000 square feet (465 square meters) of AAC masonry.

Table 1.19.3 – Level C Quality Assurance

MINIMUM TESTS				
Verification of f'_m and f'_{wc} in accordance with Article 1.4 B prior to construction and for every 5,000 sq. ft (465 sq. m) during construction				
Verification of proportions of materials in premixed of preblended mortar, prestressing grout, and grout other than self-consolidating grout, as delivered to the project site				
Verification of Slump flow and Visual Stability Index (VSI) as delivered to the project site in accordance with Article 1.5 B.1.b.3 for self-consolidating grout.				
MINIMUM INSPECTION				
Inspection Task	Frequency ^(a)		Reference for Criteria	
	Continuous	Periodic	TMS 402 ACI 530/ ASCE 5	TMS 602/ ACI 530.1/ ASCE 6
1. Verify compliance with the approved submittals		X		Art. 1.5
2. Verify that the following are in compliance:				
a. Proportions of site-mixed mortar, grout and prestressing grout for bonded tendons		X		Art. 2.1, 2.6 A, 2.6 B, 2.6 C, 2.4 G.1.b
b. Grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages		X	Sec. 1.16	Art. 2.4, 3.4
c. Placement of masonry units and construction of mortar joints		X		Art. 3.3 B
d. Placement of reinforcement, connectors, and prestressing tendons and anchorages	X		Sec. 1.16	Art. 3.2 E, 3.4, 3.6 A
e. Grout space prior to grouting	X			Art. 3.2 D 3.2 F
f. Placement of grout and prestressing grout for bonded tendons	X			Art. 3.5, 3.6 C
g. Size and location of structural elements		X		Art. 3.3 F
h. Type, size and location of anchors including other details of anchorage of masonry to structural members, frames, or other construction	X		Sec. 1.16.4.3 1.17.1	
i. Welding of reinforcement	X		Sec. 2.1.8.7.2, 3.3.3.4 (c) 8.3.3.4 (b)	
j. Preparation, construction and protection of masonry during cold weather (temperature below 40°F) (4.4°C) or hot weather (temperature above 90°F) (32.2°C)		X		Art. 1.8 C, 1.8 D
k. Application and measurement of prestressing force.	X			Art. 3.6 B
l. Placement of AAC masonry units and construction of thin-bed mortar joints	X			Art. 3.3 B.8
m. Properties of thin-bed mortar for AAC masonry	X			Art. 2.1 C.1
5. Observe preparation of grout specimens, mortar specimens, and/or prisms	X			Art. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4

(a) Frequency refers to the frequency of inspection, which may be continuous during the task listed or periodically during the listed task, as defined in the table.

REQUIRED VERIFICATION AND INSPECTION OF SOILS		
TABLE 1705.6		
VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASKS LISTED	PERIODICALLY DURING TASK LISTED
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	---	X
2. Verify excavations are extended to proper depth and have reached proper material.	---	X
3. Perform classification and testing of compacted fill materials.	---	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	X	---
5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.	---	X

REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION	
SECTION 1705	
1705.2	Steel construction. The special inspections for steel elements of buildings and structures shall be as required in this section. Exception: Special inspection of the steel fabrication process shall not be required where the fabricator does not perform any welding thermal cutting or heating operation of any kind as part of the fabrication process. In such cases, the fabricator shall be required to submit a detailed procedure for material control that demonstrates the fabricator's ability to maintain suitable records and procedures such that, at any time during the fabrication process, the material specification, and grade for the main stress-carrying elements are capable of being determined. Mill test report shall be identifiable to the main stress-carry elements when required by the approved construction documents.
1705.2.1	Structural steel. Special inspection for structural steel shall be in accordance with the quality assurance inspection requirements of AISI 360.
1705.2.2	Steel construction other than structural steel. Special inspection for steel construction other than structural steel shall be in accordance with Table 1705.2.2 and this section.
1705.2.2.1	Welding. Welding inspection and Welding inspector qualification shall be in accordance with this section.
1705.2.2.1.1	Cold-formed steel. Welding inspection and Welding inspector qualification Cold-formed steel floor and roof decks shall be in accordance with AWS D1.3.
1705.2.2.1.2	Reinforcing steel. Welding inspection and Welding inspector qualification for reinforcing steel shall be in accordance with AWS D1.4 and ACI 318.
1705.2.2.2	Cold-formed steel trusses spanning 60 feet or greater. Where a Cold-formed steel truss clear span is 60 feet (18 288 mm) or greater, the special inspector shall verify that the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing are installed in accordance with the approved truss submittal package,

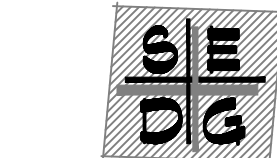
TABLE 1705.2.2
REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL

VERIFICATION AND INSPECTION	CONTINUOUS		PERIODIC	REFERENCED STANDARD ^a
1. Material verification of cold-formed steel deck:				
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	---		X	Applicable ASTM material standards
b. Manufacturer's certified test reports.	---		X	
2. Inspection of welding:				
a. Cold-formed steel deck:				
1) Floor and roof deck welds.	---		X	AWS D1.3
b. Reinforcing steel:				
1) Verification of weldability of reinforcing steel other than ASTM A 706	---		X	AWS D1.4 ACI 318: Section 3.5.2
2) Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.	X		---	
3) Shear reinforcement.		X	---	
4) Other reinforcing steel.		---	X	

For SI: 1 inch = 25.4 mm.
a. Where applicable, see also Section 1705.11, Special inspections for seismic resistance.

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION				
TABLE 1705.3				
VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD ^a	IBC REFERENCE
1. Inspection of reinforcing steel, including prestressing tendons, and placement.	---	X	ACI 318: 3.5, 7.1–7.7	1910.4
2. Inspection of reinforcing steel welding in accordance with Table 1705.2.2, Item 2b.	---	---	AWS D1.4 ACI 318: 3.5.2	---
3. Inspection of anchors cast in concrete where allowable loads have been increased or where strength design is used.	---	X	ACI 318: 8.1.3, 21.2.8	1908.5, 1909.1
4. Inspection of anchors post-installed in hardened concrete members ^b	---	X	ACI 318: 3.8.6, 8.1.3, 21.2.8	1909.1
5. Verifying use of required design mix.	---	X	ACI 318: Ch. 4, 5.2–5.4	1904.2, 1910.2 1910.3
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	---	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1910.10
7. Inspection of concrete and shotcrete placement for proper application techniques.	X	---	ACI 318: 5.9, 5.10	1910.6, 1910.7 1910.8
8. Inspection for maintenance of specified curing temperature and techniques.	---	X	ACI 318: 5.11–5.13	1910.9
9. Inspection of prestressed concrete: a. Application of prestressing forces. b. Grouting of bonded prestressing tendons in the seismic force-resisting system.	X X	---	ACI 318: 18.20 ACI 318: 18.18.4	---
10. Erection of precast concrete members.	---	X	ACI 318: Ch. 16	---
11. Verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	---	X	ACI 318: 6.2	---
12. Inspect formwork for shape, location and dimensions of the concrete member being formed.	---	X	ACI 318: 6.1.1	---

For SI: 1 inch = 25.4 mm.
a. Where applicable, see also Section 1705.11, Special inspections for seismic resistance.



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AUSTIN, TEXAS

A Development By [LDG]
Architecture By [KELLY GROSSMAN]

INSPECTIONS

Sheet Title

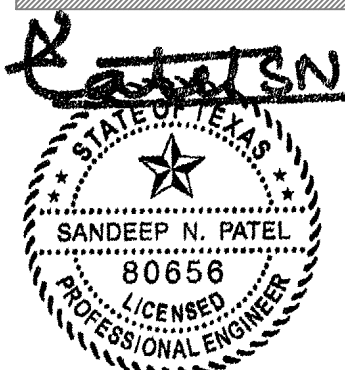
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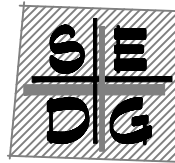
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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

OVERALL FOUNDATION PLAN

BUILDING TYPE I

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Description

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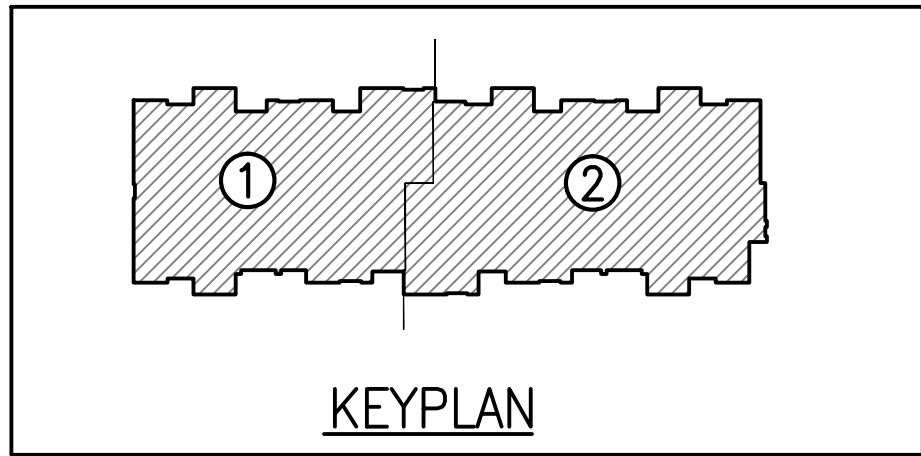
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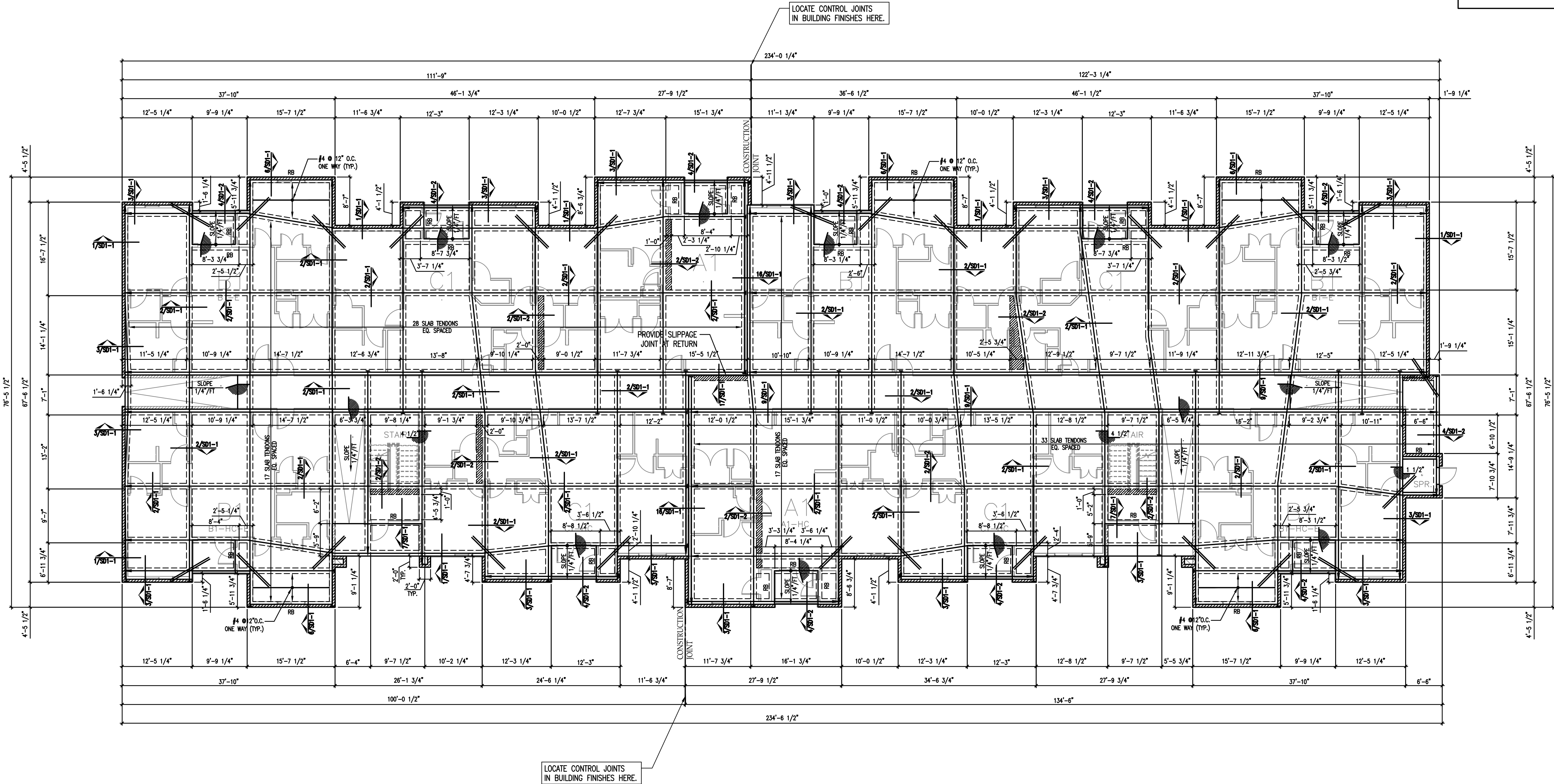
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S1-1

PLAN



KEYPLAN



1 OVERALL FOUNDATION PLAN – BUILDING TYPE I
SCALE: 3/32"=1'-0"

RE: S0-1, S0-2 & S0-5
FOR FOUNDATION NOTES
& SCHEDULE

PLEASE REVIEW
ARCHITECTURAL DRGS. FOR
DIMENSIONS, SLOPES,
DROPS & DEPRESSIONS.

SLAB NOTE

"4" THICK CONCRETE SLAB (UNLESS NOTED OTHERWISE)
ON MINIMUM TEN (10) MIL VAPOR BARRIER ON COMPACTED FILL
SUBGRADE. THE VAPOR BARRIER SHALL CONFORM TO ASTM
E1745 CLASS A OR BETTER AND SHALL HAVE A MAXIMUM WATER
VAPOR PERMEANCE OF 0.01 PERMS WHEN TESTED IN
ACCORDANCE WITH ASTM E96.
THE VAPOR BARRIER SHALL BE INSTALLED PER THE
MANUFACTURER'S RECOMMENDATIONS AND ASTM E1643.
STANDARD PRACTICE FOR INSTALLATION OF WATER VAPOR
BARRIERS USED IN CONTACT WITH EARTH OR GRANULAR FILL
UNDER CONCRETE SLABS. REFER TO GEOTECHNICAL REPORT FOR
SUBGRADE PREPARATION AND COMPACTION REQUIREMENTS.

GRADE BEAM SCHEDULE

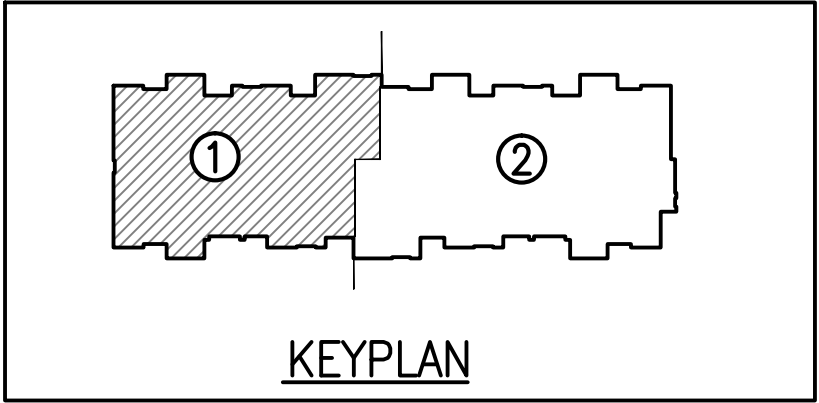
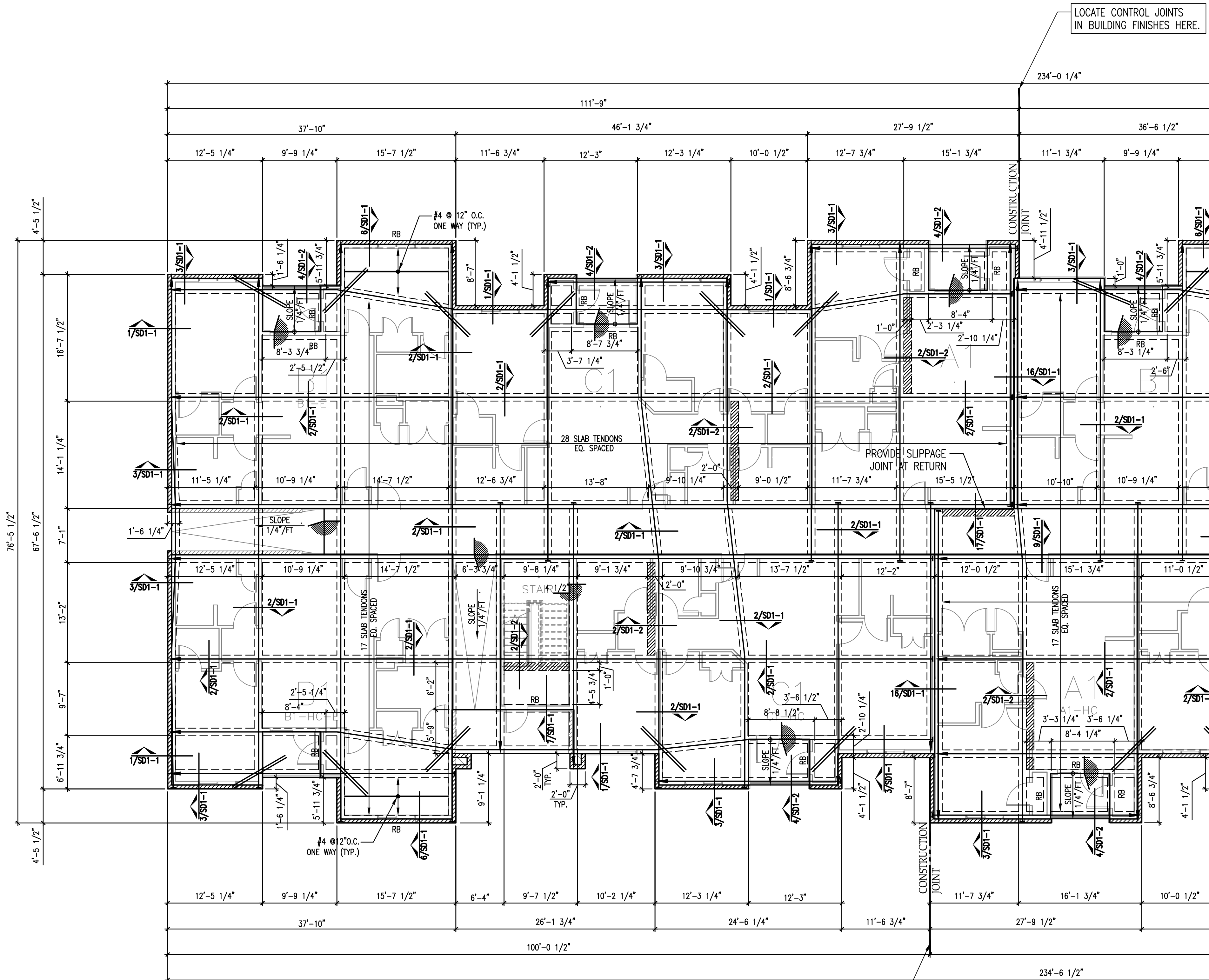
PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB		RE-PLAN	12"	2-#5 BOTT.
REINF. BEAM		12"	30"	2-#5 BOTT. w/#3 TIES@EACH TENDON
GRADE BEAM		12"	30"	1 TENDON

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1

PARTIAL FOUNDATION PLAN - BUILDING TYPE I (AREA #1)

SCALE: 1/8"=1'-0"



RE: S0-1, S0-2 & S0-5
FOR FOUNDATION NOTES
& SCHEDULE

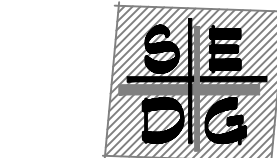
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SLAB NOTE

4" THICK CONCRETE SLAB (UNLESS NOTED OTHERWISE)
ON MINIMUM TEN (10) MIL VAPOR BARRIER ON COMPACTED FILL
SUBGRADE. THE VAPOR BARRIER SHALL CONFORM TO ASTM
E1745 CLASS A OR BETTER AND SHALL HAVE A MAXIMUM WATER
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SUBGRADE PREPARATION AND COMPACTION REQUIREMENTS.

GRADE BEAM SCHEDULE

PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB		RE-PLAN	12"	2-#5 BOTT.
REINF. BEAM		12"	30"	2-#5 BOTT. w/#3 TIES@EACH TENDON
GRADE BEAM		12"	30"	1 TENDON



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Architecture By [KELLY GROSSMAN]

PARTIAL FOUNDATION PLAN
BUILDING TYPE I - AREA #1

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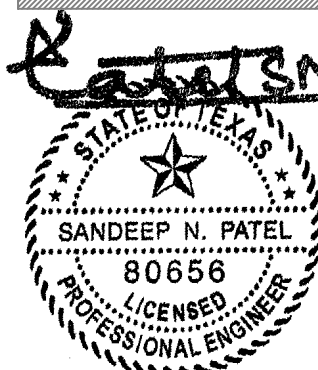
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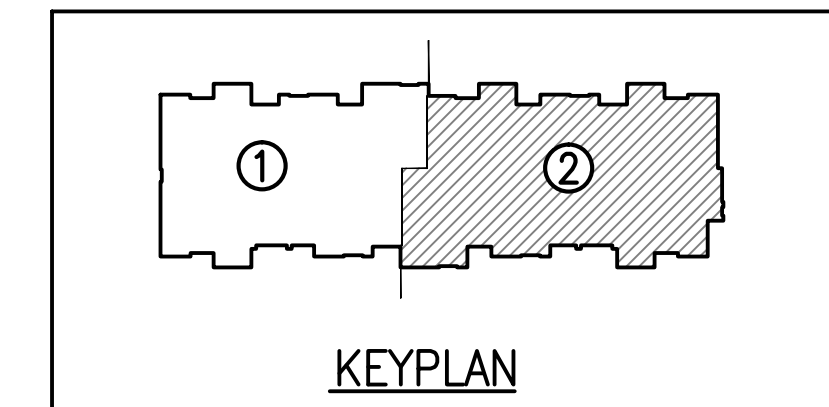
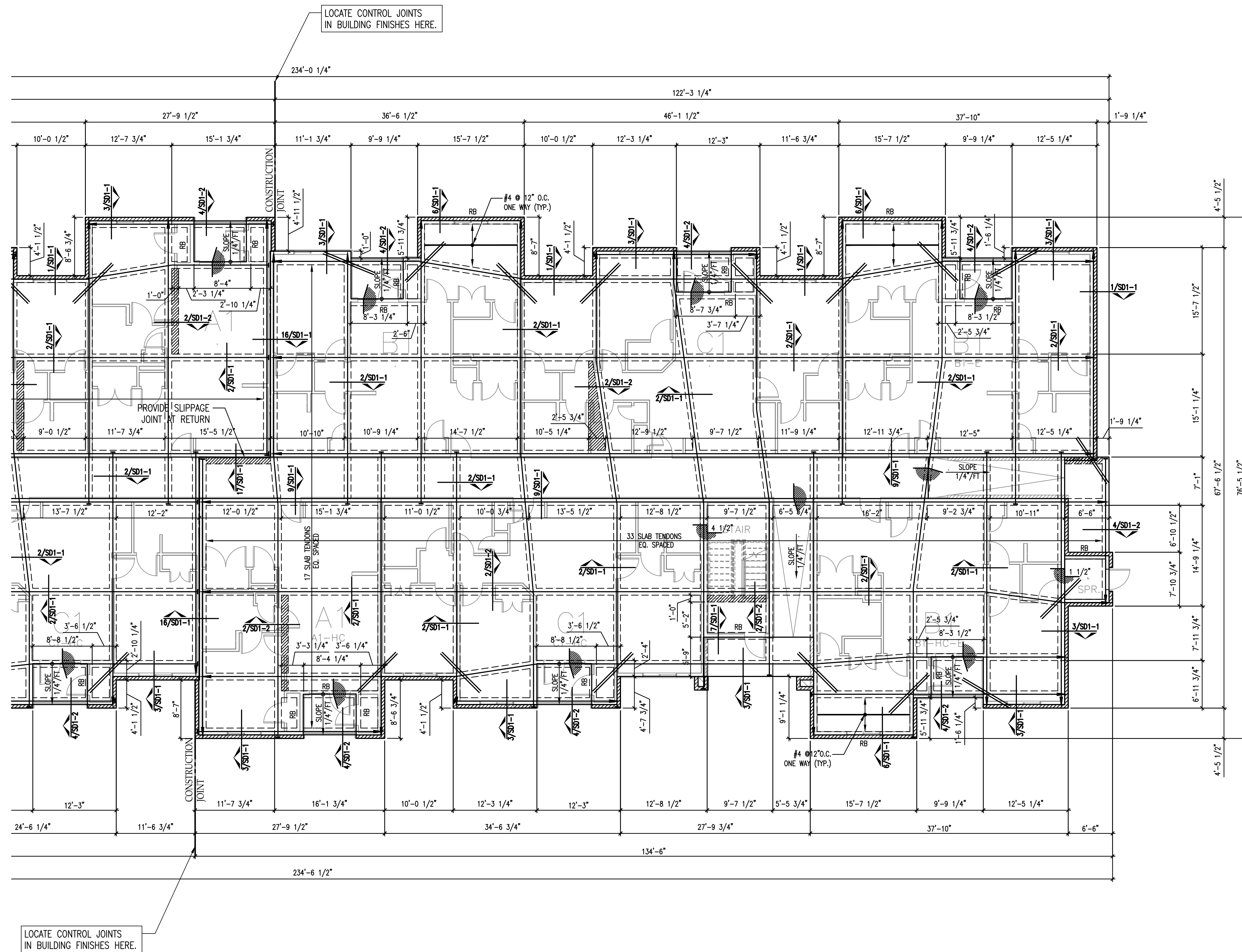
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BUILDING TYPE 1 - AREA #2

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Sandeep N. Patel



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PLAN

1 PARTIAL FOUNDATION PLAN – BUILDING TYPE I (AREA #2)
SCALE: 1/8"=1'-0"




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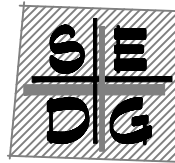
SLAB NOTE

"4" THICK CONCRETE SLAB (UNLESS NOTED OTHERWISE)
ON MINIMUM TEN (10) MIL VAPOR BARRIER OR COMPACTED FINE GRADED SAND OR GRAVEL VAPOR BARRIER TO CONFORM TO ASTM E1745 CLASS A OR BETTER AND SHALL HAVE A MAXIMUM WATER VAPOR PERMEANCE OF 0.01 PERMS WHEN TESTED IN ACCORDANCE WITH ASTM E96.
THE VAPOR BARRIER SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS AND ASTM E1643, STANDARD PRACTICE FOR INSTALLATION OF WATER VAPOR BARRIERS USED IN CONTACT WITH EARTH OR GRANULAR FILL UNDER CONCRETE SLABS. REFER TO GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION AND COMPACTION REQUIREMENTS.

GRADE BEAM SCHEDULE

PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB		RE:PLAN	12"	2-#5 BOTT.
REINF. BEAM		12"	30"	2-#5 BOTT. w/#3 TIES@EACH TENDON
GRADE BEAM		12"	30"	1 TENDON

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OVERALL FOUNDATION PLAN

BUILDING TYPE II

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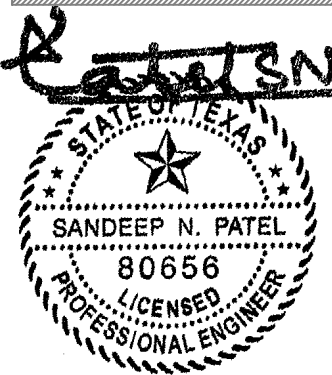
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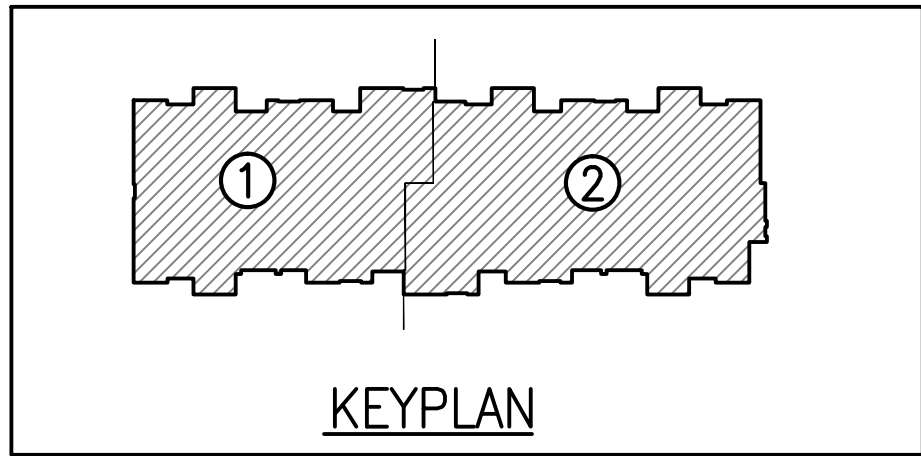
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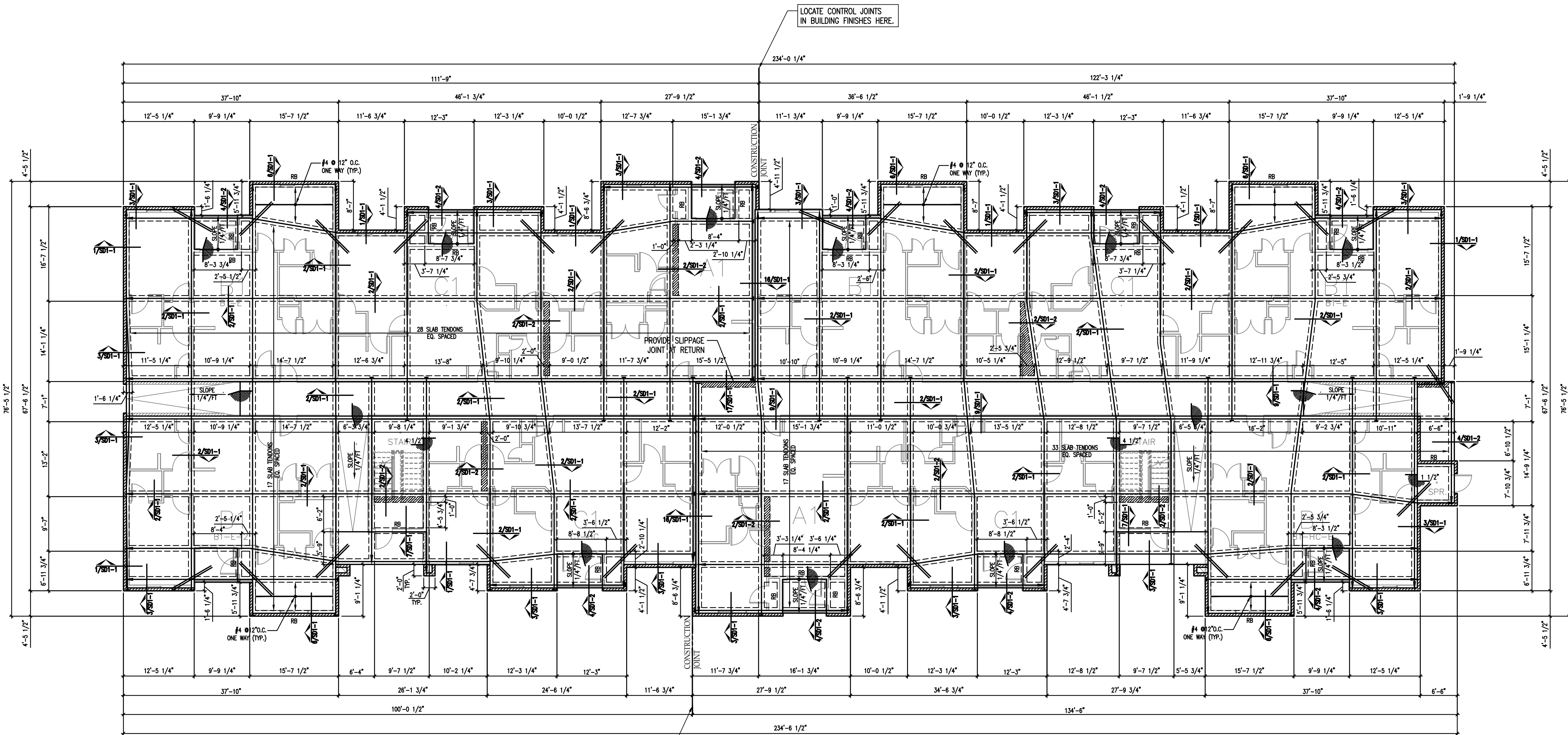
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S1-2

PLAN



KEYPLAN



1 OVERALL FOUNDATION PLAN - BUILDING TYPE II
SCALE: 3/32\"/>




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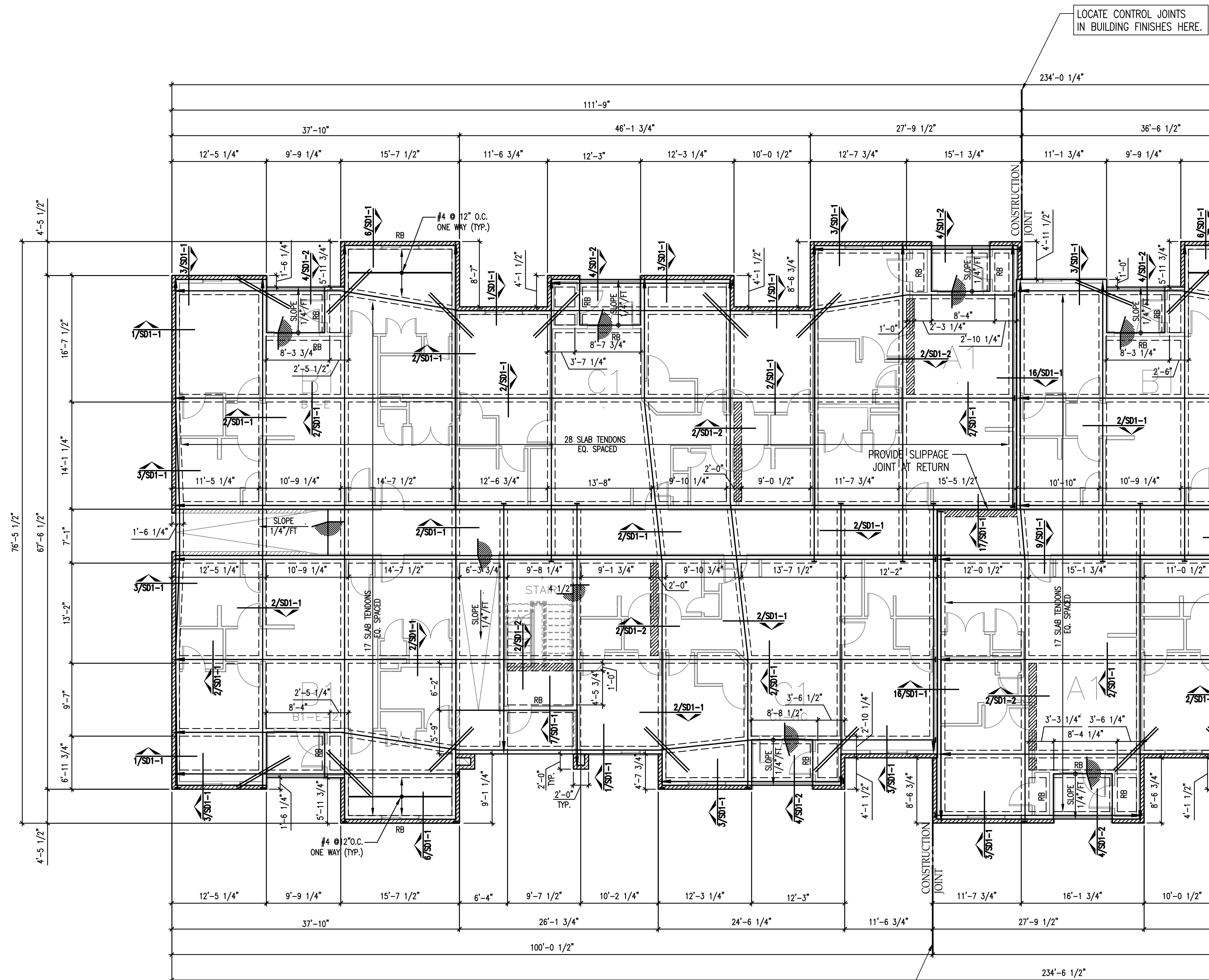
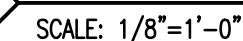
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SLAB NOTE

*4\"/>

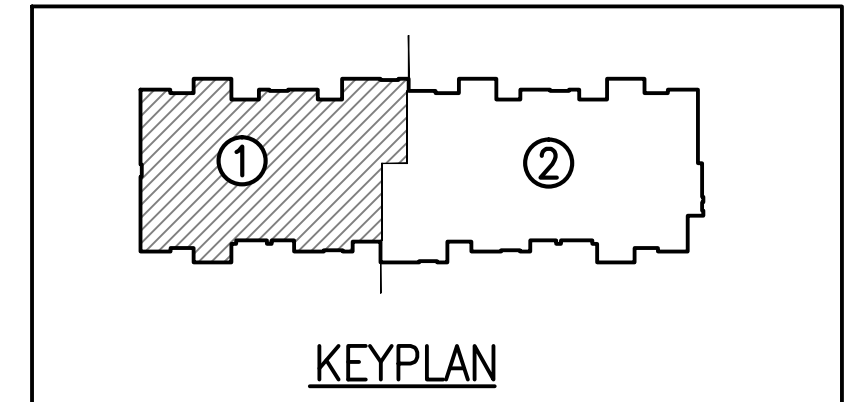
GRADE BEAM SCHEDULE

PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB		RE:PLAN	12"	2-#5 BOTT.
REINF. BEAM		12"	30"	2-#5 BOTT. w/#3 TIES@EACH TENDON
GRADE BEAM		12"	30"	1 TENDON



LOCATE CONTROL JOINTS
IN BUILDING FINISHES HERE.

LOCATE CONTROL JOINTS
IN BUILDING FINISHES HERE.






RE: S0-1, S0-2 & S0-5
FOR FOUNDATION NOTES
& SCHEDULE

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DROPS & DEPRESSIONS.

SLAB NOTE

"4" THICK CONCRETE SLAB (UNLESS NOTED OTHERWISE)
ON MINIMUM TEN (10) MIL VAPOR BARRIER ON COMPACTED FILL SUBGRADE. THE VAPOR BARRIER SHALL CONFORM TO ASTM E1745 CLASS A OR BETTER AND SHALL HAVE A MAXIMUM WATER VAPOR PERMEANCE OF 0.01 PERMS WHEN TESTED IN ACCORDANCE WITH ASTM E96.

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GRADE BEAM SCHEDULE				
PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB		RE:PLAN	12"	2-#5 BOT.
REINF. BEAM		12"	30"	2-#5 BOT. w/#3 TIES EACH TENDON
GRADE BEAM		12"	30"	1 TENDON

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Handwritten: ~~Abhishek~~ SN



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SANDEEP N. PATEL
80656
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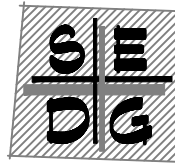
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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

OVERALL FOUNDATION PLAN
BUILDING TYPE III

Sheet Title:

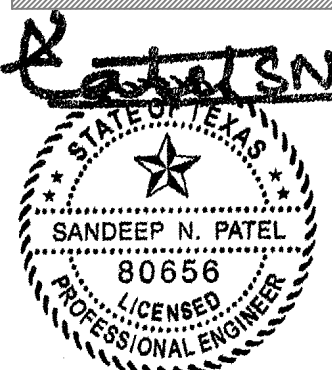
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Description

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☐ Construction



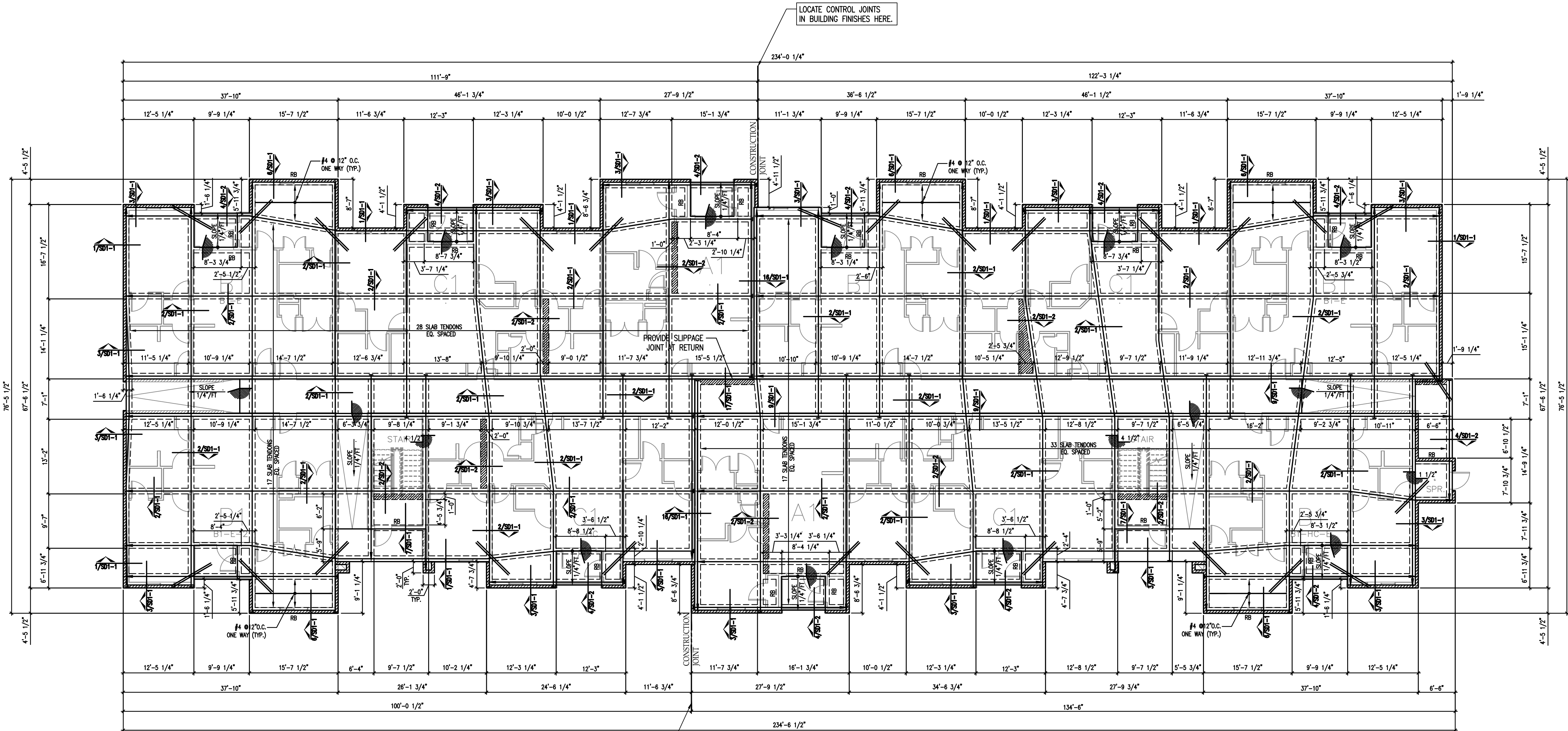
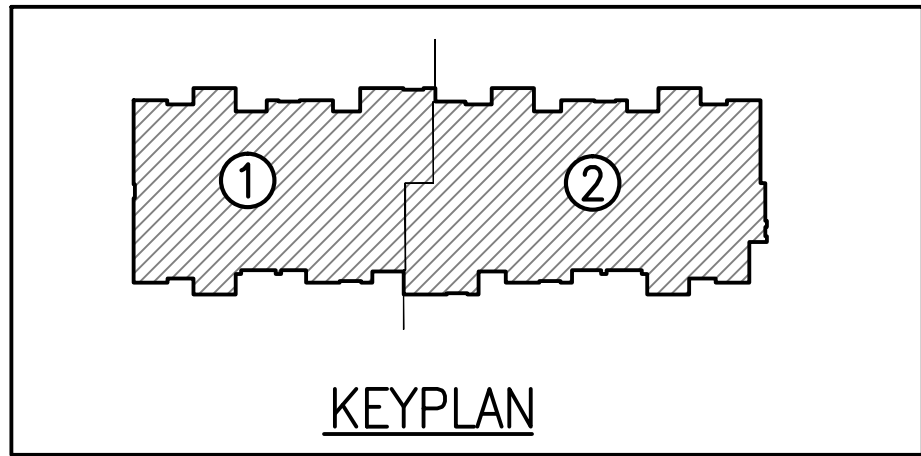
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SHEET NO.

S1-3

PLAN



1 OVERALL FOUNDATION PLAN - BUILDING TYPE III

SCALE: 3/32"=1'-0"




RE: S0-1, S0-2 & S0-5
FOR FOUNDATION NOTES
& SCHEDULE

PLEASE REVIEW
ARCHITECTURAL DRGS. FOR
DIMENSIONS, SLOPES,
DROPS & DEPRESSIONS.

SLAB NOTE

"4" THICK CONCRETE SLAB (UNLESS NOTED OTHERWISE)
ON MINIMUM TEN (10) MIL VAPOR BARRIER ON COMPACTED FILL
SUBGRADE. THE VAPOR BARRIER SHALL CONFORM TO ASTM
E1745 CLASS A OR BETTER AND SHALL HAVE A MAXIMUM WATER
VAPOR PERMEANCE OF 0.01 PERMS WHEN TESTED IN
ACCORDANCE WITH ASTM E96.
THE VAPOR BARRIER SHALL BE INSTALLED PER THE
MANUFACTURER'S RECOMMENDATIONS AND ASTM E1643,
STANDARD PRACTICE FOR INSTALLATION OF WATER VAPOR
BARRIERS USED IN CONTACT WITH EARTH OR GRANULAR FILL
UNDER CONCRETE SLABS. REFER TO GEOTECHNICAL REPORT FOR
SUBGRADE PREPARATION AND COMPACTION REQUIREMENTS.

GRADE BEAM SCHEDULE

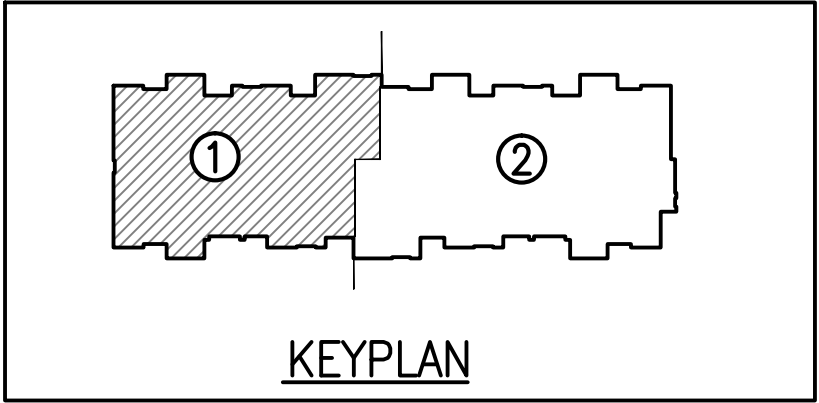
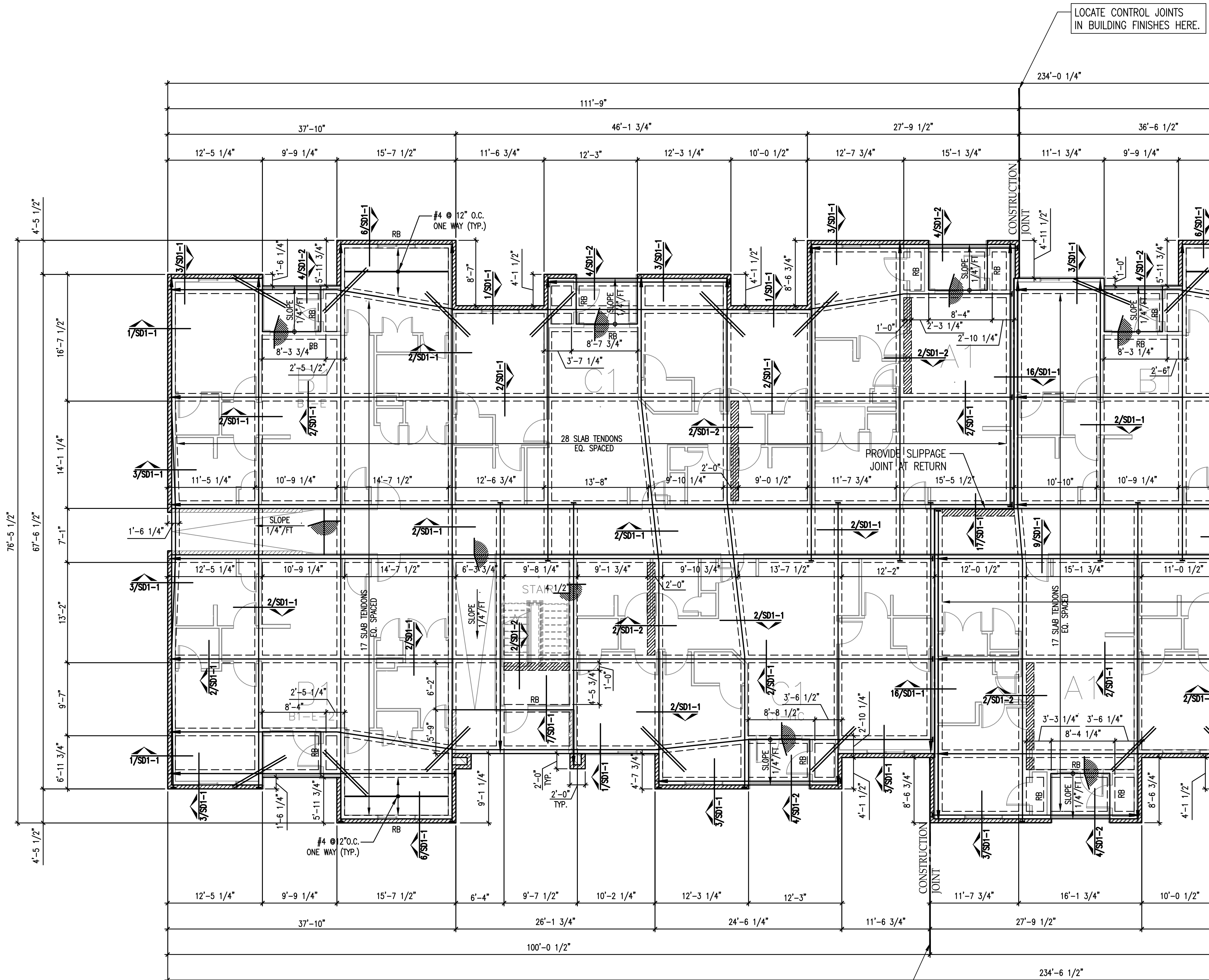
PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB		RE:PLAN	12"	2-#5 BOTT.
REINF. BEAM		12"	30"	2-#5 BOTT. w/#3 TIES@EACH TENDON
GRADE BEAM		12"	30"	1 TENDON

G:\\$136 - KELLY GROSSMAN\136-091 Moonlight Ranch\Structural Drawings\Working Drawings\31-3-1.dwg Plotted: June 10, 2019 - 11:02 AM by Hao Tran

1

PARTIAL FOUNDATION PLAN - BUILDING TYPE III (AREA #1)

SCALE: 1/8"=1'-0"



RE: S0-1, S0-2 & S0-5
FOR FOUNDATION NOTES
& SCHEDULE

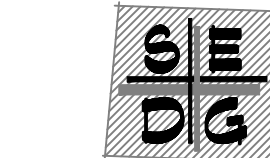
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ARCHITECTURAL DRGS. FOR
DIMENSIONS, SLOPES,
DROPS & DEPRESSIONS.

SLAB NOTE

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ON MINIMUM TEN (10) MIL VAPOR BARRIER ON COMPACTED FILL
SUBGRADE. THE VAPOR BARRIER SHALL CONFORM TO ASTM
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VAPOR PERMANENCE OF 0.01 PERMS WHEN TESTED IN
ACCORDANCE WITH ASTM E96.
THE VAPOR BARRIER SHALL BE INSTALLED PER THE
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STANDARD PRACTICE FOR INSTALLATION OF WATER VAPOR
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UNDER CONCRETE SLABS. REFER TO GEOTECHNICAL REPORT FOR
SUBGRADE PREPARATION AND COMPACTION REQUIREMENTS.

GRADE BEAM SCHEDULE

PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB	RE-PLAN	12"	30"	2-#5 BOT.
REINF. BEAM	RB	12"	30"	2-#5 BOT. w/#3 TIES@EACH TENDON
GRADE BEAM	RB	12"	30"	1 TENDON



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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

PARTIAL FOUNDATION PLAN
BUILDING TYPE III - AREA #1

Sheet Title:

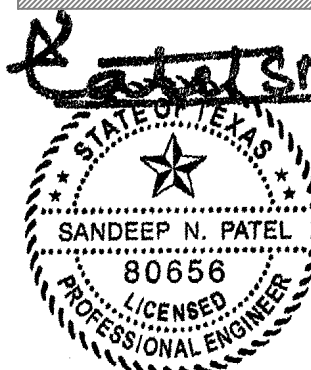
Date

Description

Rev.

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Checked By: DMH/ZA
Drawing Scale: As Noted
Project No. 136-091

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☐ CD 95%
☐ CD 100%
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☐ Construction



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S1-3.1

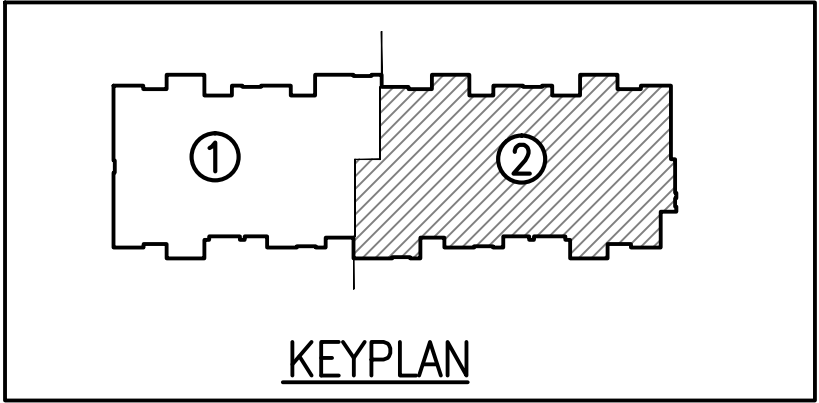
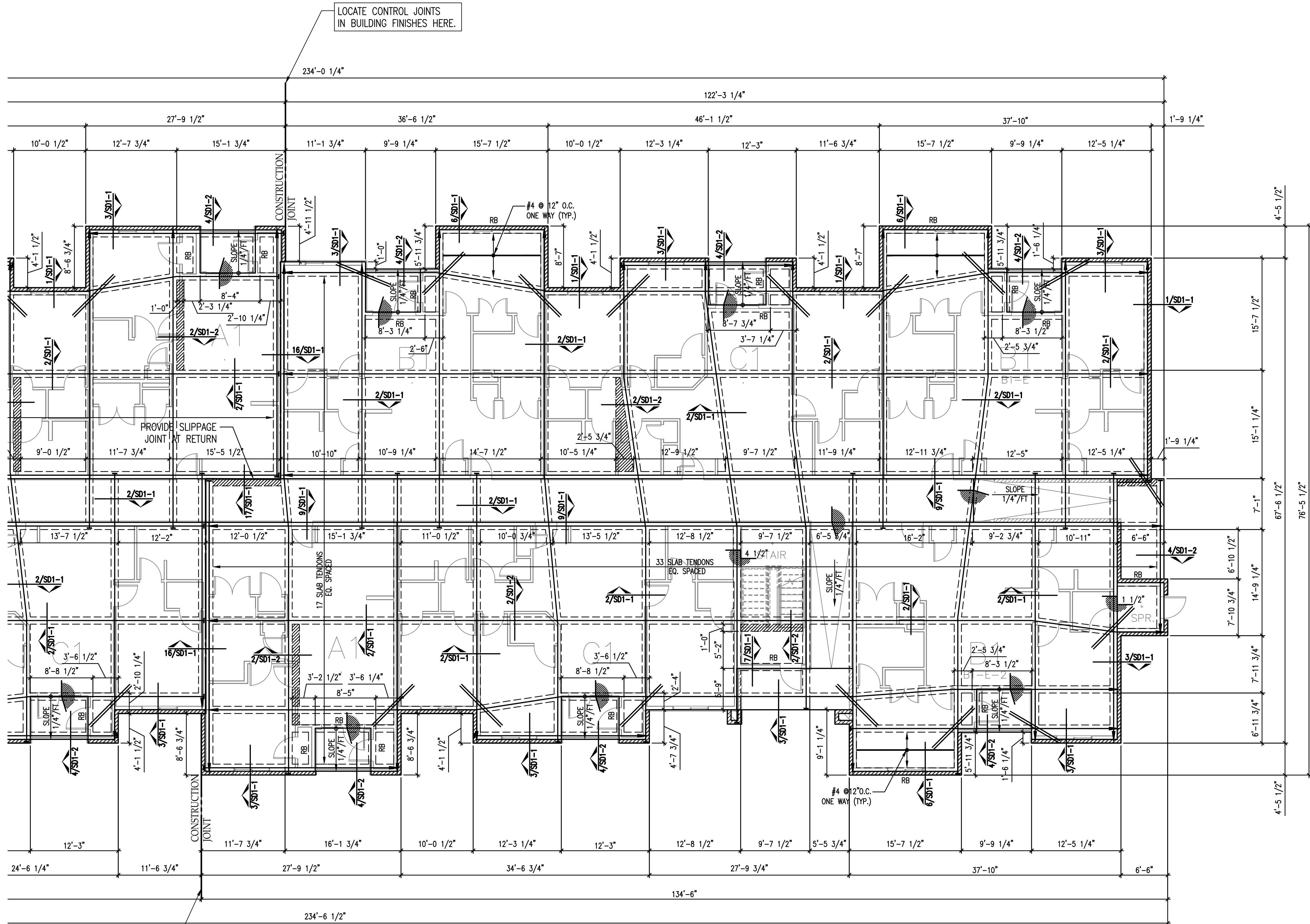
PLAN

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1

PARTIAL FOUNDATION PLAN - BUILDING TYPE III (AREA #2)

SCALE: 1/8"=1'-0"



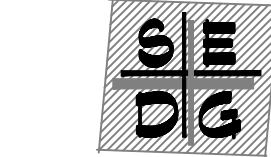
RE: S0-1, S0-2 & S0-5
FOR FOUNDATION NOTES
& SCHEDULE

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DROPS & DEPRESSIONS.

SLAB NOTE

4" THICK CONCRETE SLAB (UNLESS NOTED OTHERWISE)
ON MINIMUM TEN (10) MIL VAPOR BARRIER ON COMPACTED FILL
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UNDER CONCRETE SLABS. REFER TO GEOTECHNICAL REPORT FOR
SUBGRADE PREPARATION AND COMPACTION REQUIREMENTS.

GRADE BEAM SCHEDULE				
PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB	RE-PLAN	12"	30"	2-#5 BOTT.
REINF. BEAM	RB	12"	30"	2-#5 BOTT.
GRADE BEAM	---	12"	30"	1 TENDON



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PARTIAL FOUNDATION PLAN
BUILDING TYPE III - AREA #2

Sheet Title:

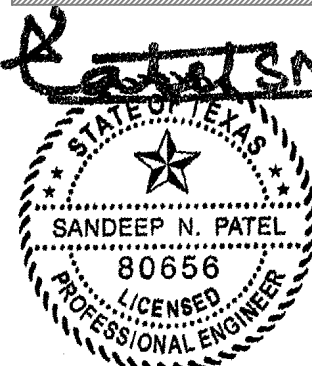
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S1-3.2

PLAN

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]


A Development By [LDG]
Architecture By [KELLY GROSSMAN]

Sheet Title: **2ND FLOOR FRAMING PLAN - BUILDING TYPE I**

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Drawing Scale:	Project No.
As Noted	136-091

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<input type="checkbox"/> CD 95%		_____
<input type="checkbox"/> CD 100%		_____
<input type="checkbox"/> Pricing		_____
<input type="checkbox"/> Bidding		_____
<input type="checkbox"/> Permit		_____
<input type="checkbox"/> Construction		_____

2 Patel SN



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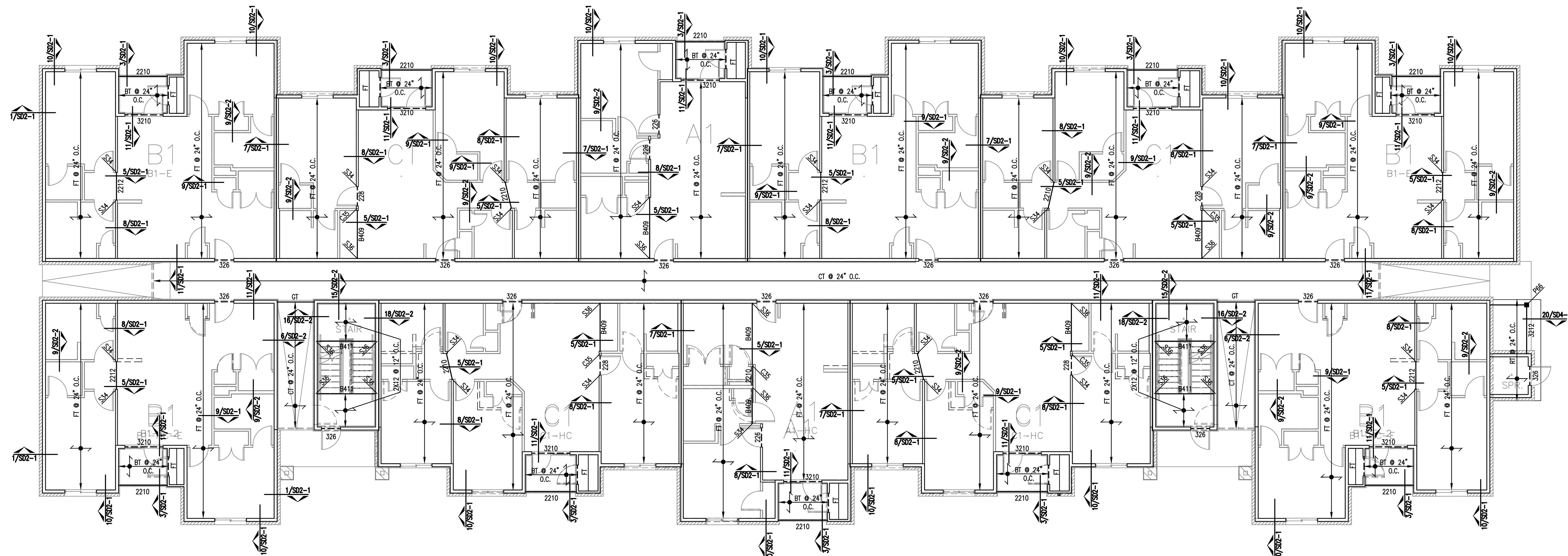
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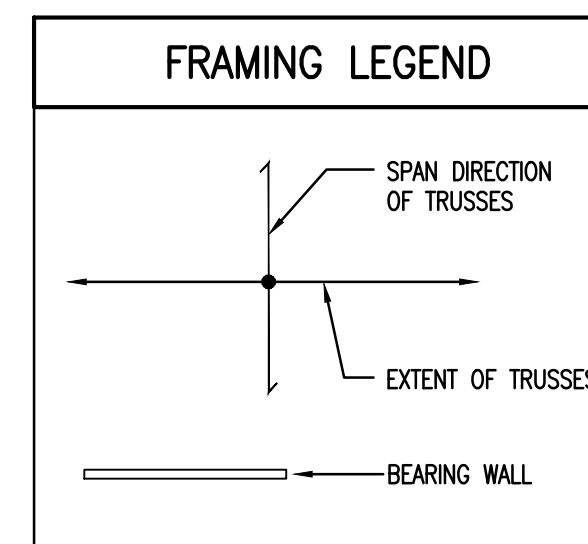
S2-1

PLAN



1 2ND FLOOR FRAMING PLAN – BLDG TYPE I
SCALE: 1/8"=1'-0"

SCALE: 1/8"=1'-0"



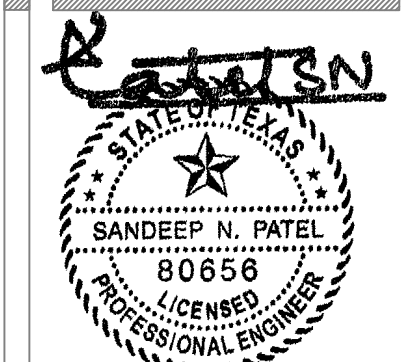
RE: S0-1, S0-3 & S0-5
FOR FRAMING NOTES
AND SCHEDULE

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]
Sheet Title
3RD FLOOR FRAMING PLAN - BUILDING TYPE I

[illegible]

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Drawing Scale:	Project No.
As Noted	136-091

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<input type="checkbox"/> Coordination		_____
<input type="checkbox"/> CD 95%		_____
<input type="checkbox"/> CD 100%		_____
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<input type="checkbox"/> Bidding		_____
<input type="checkbox"/> Permit		_____
<input type="checkbox"/> Construction		_____



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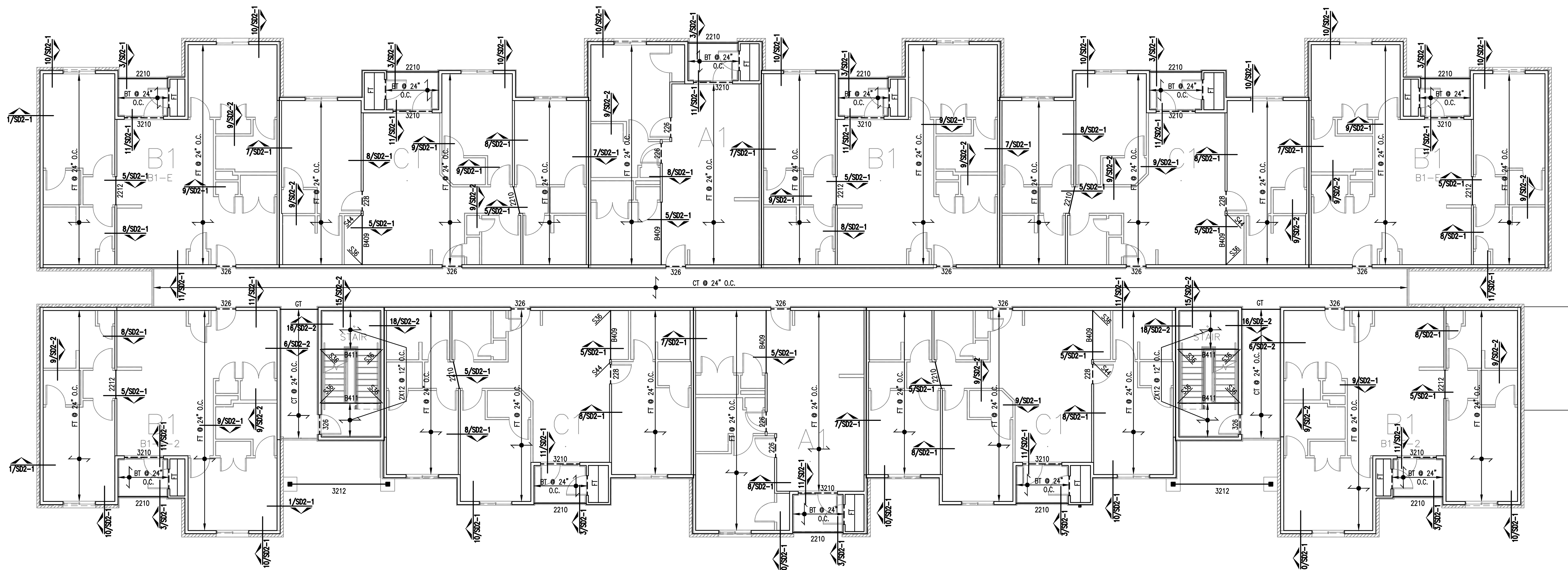
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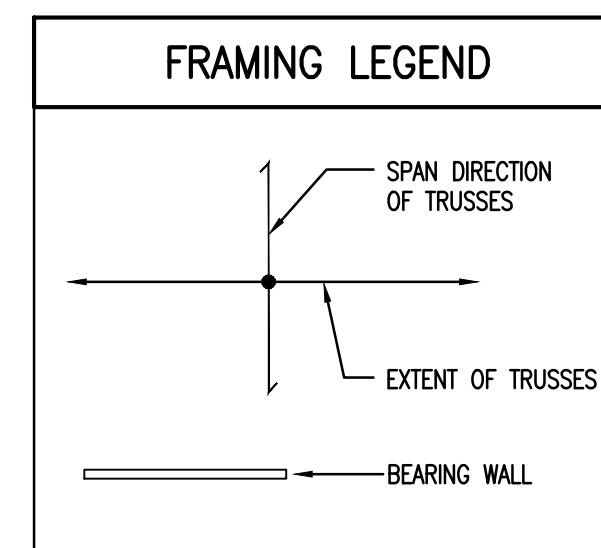
SHEET NO.

S2-1A

PLAN



1 3RD FLOOR FRAMING PLAN - BLDG TYPE I
SCALE: 1/8"=1'-0"



RE: S0-1, S0-3 & S0-5
FOR FRAMING NOTES
AND SCHEDULE

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

[illegible]

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<input type="checkbox"/> Coordination		_____
<input type="checkbox"/> CD 95%		_____
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<input type="checkbox"/> Bidding		_____
<input type="checkbox"/> Permit		_____
<input type="checkbox"/> Construction		_____

Handwritten: 2013 01/13/2013

Seal: STATE OF TEXAS
SANDEEP N. PATEL
80656
LICENSED
PROFESSIONAL ENGINEER

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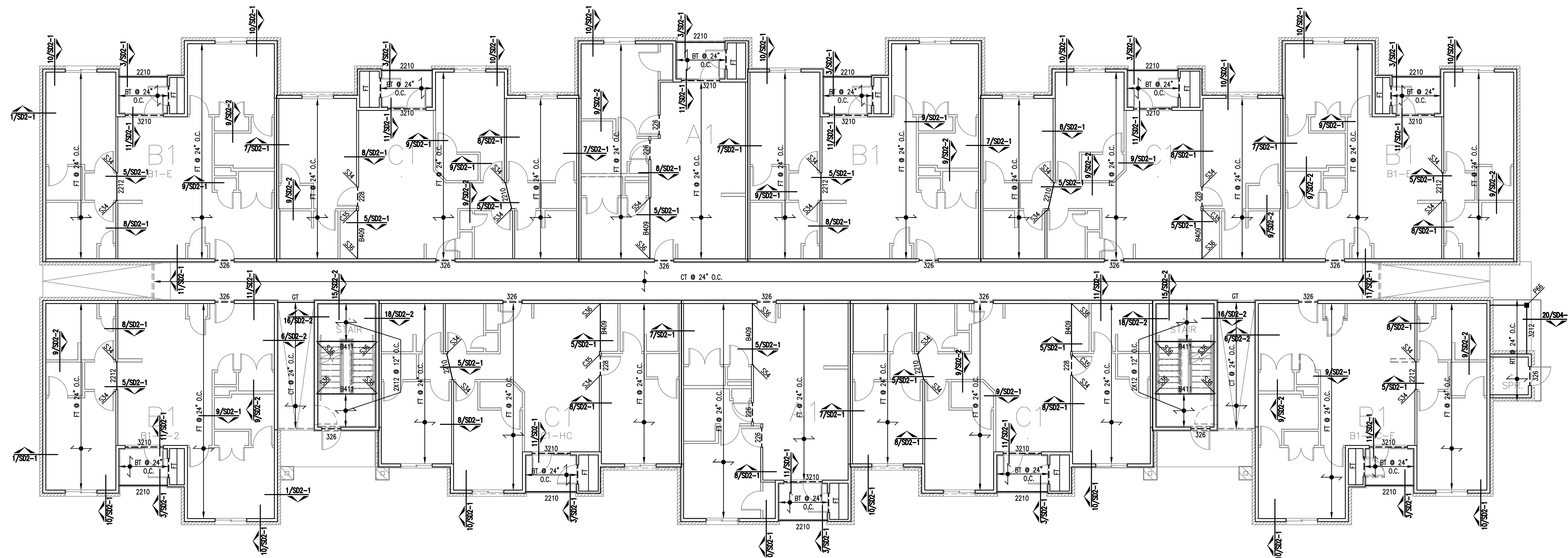
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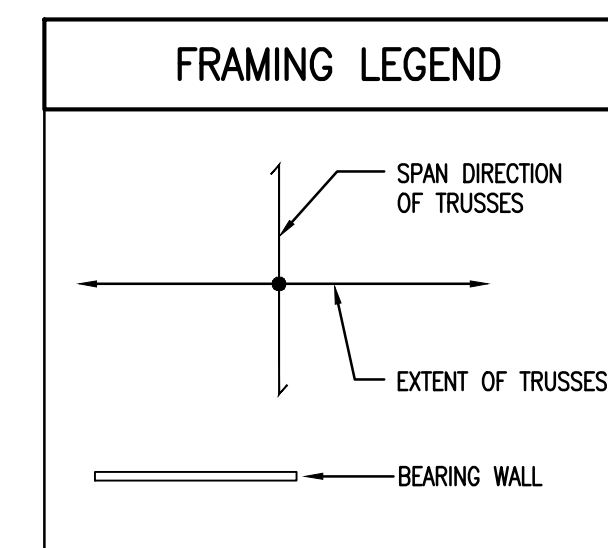
SHEET NO.

S2-2

PLAN



1 2ND FLOOR FRAMING PLAN – BUILDING TYPE II
SCALE: 1/8"=1'-0"



RE: SO-1, SO-3 & SO-5
FOR FRAMING NOTES
AND SCHEDULE

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

Sheet Title: **3RD FLOOR FRAMING PLAN - BUILDING TYPE II**


Sheet Title: **3RD FLOOR FRAMING PLAN - BUILDING TYPE II**

[illegible]

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HT	DVH/ZA
Drawing Scale:	Project No.
As Noted	136-091

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<input type="checkbox"/> Pricing		_____
<input type="checkbox"/> Bidding		_____
<input type="checkbox"/> Permit		_____
<input type="checkbox"/> Construction		_____

2 Patel SN



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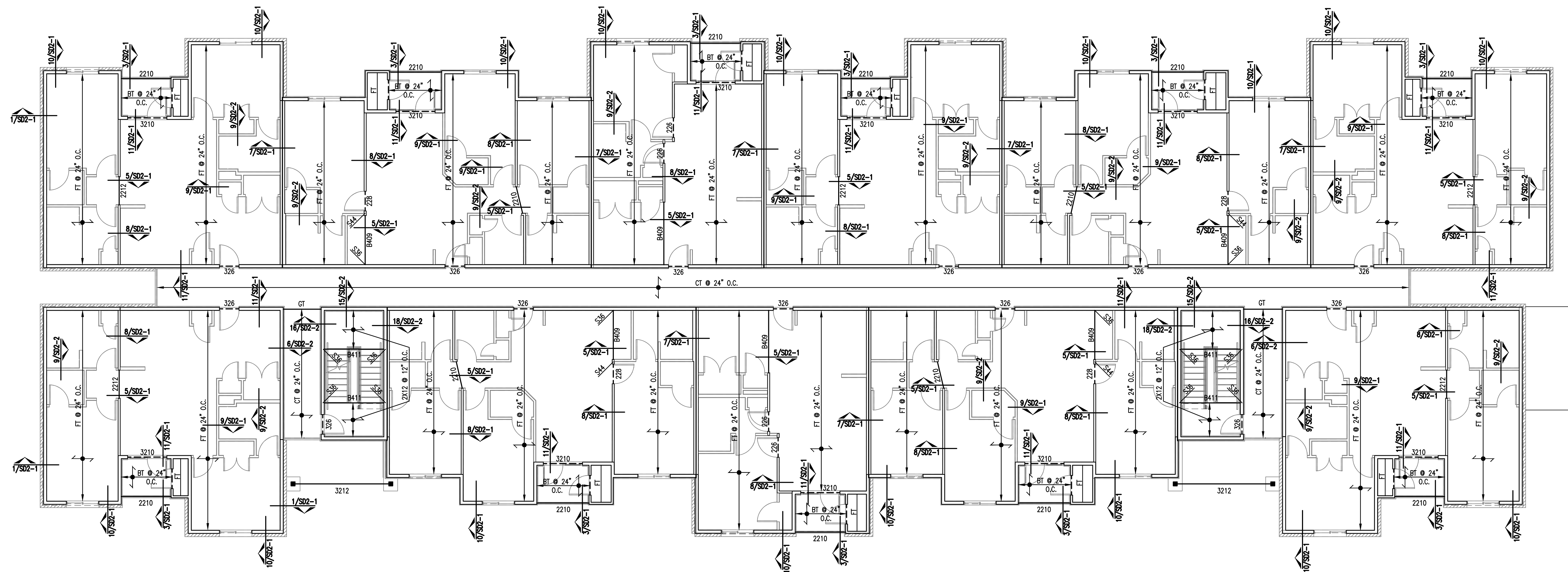
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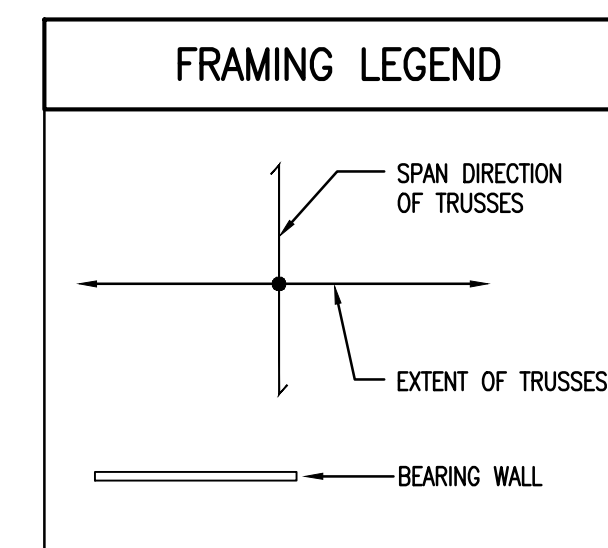
SHEET NO.

S2-2A

PLAN



1 3RD FLOOR FRAMING PLAN – BUILDING TYPE II
SCALE: 1/8"=1'-0"



RE: SO-1, SO-3 & SO-5
FOR FRAMING NOTES
AND SCHEDULE


MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

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Drawing Scale:	Project No.
As Noted	136-091

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<input type="checkbox"/> Coordination		_____
<input type="checkbox"/> CD 95%		_____
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<input type="checkbox"/> Permit		_____
<input type="checkbox"/> Construction		_____

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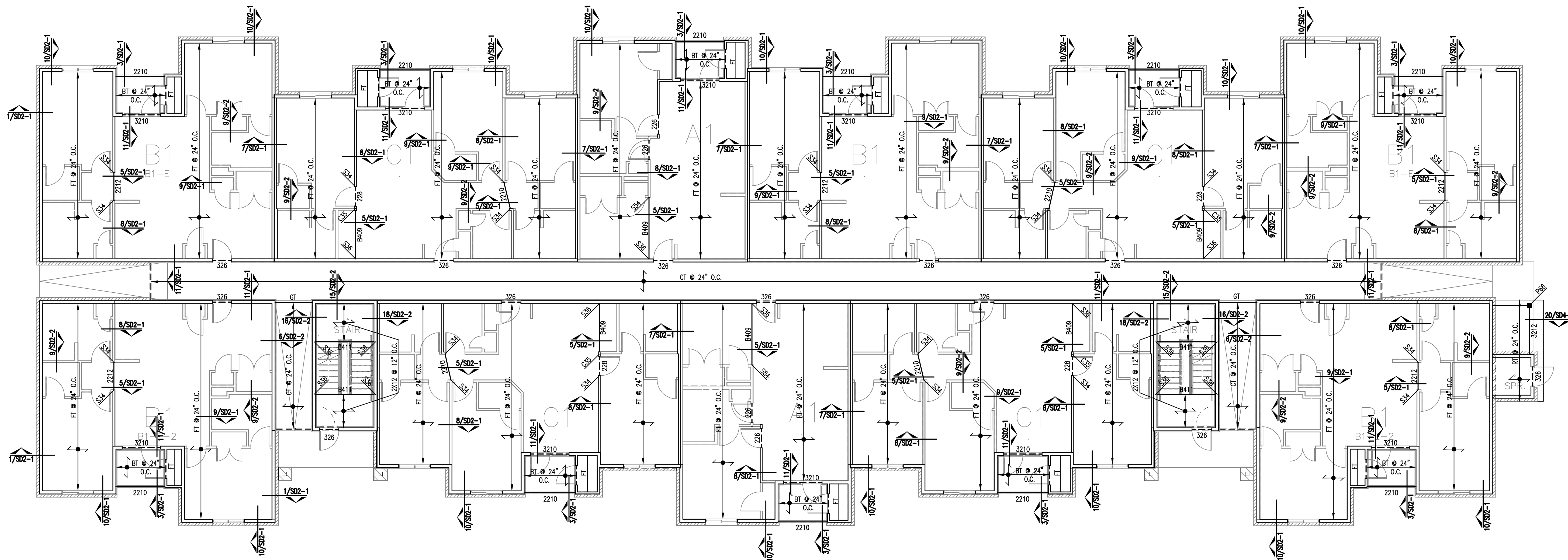
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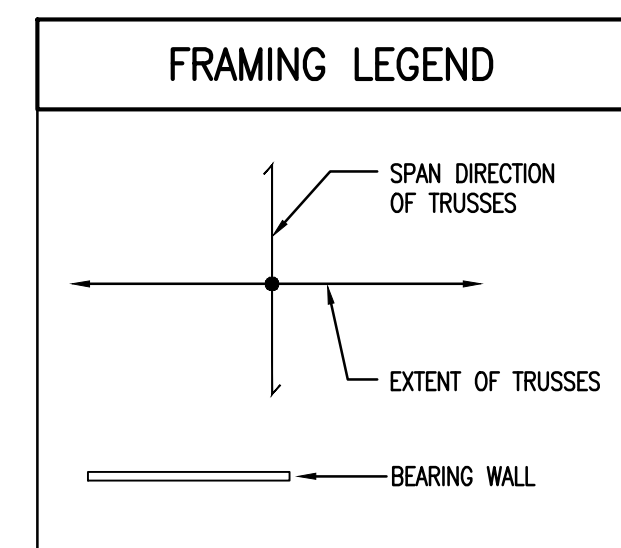
SHEET NO.

S2-3

PLAN

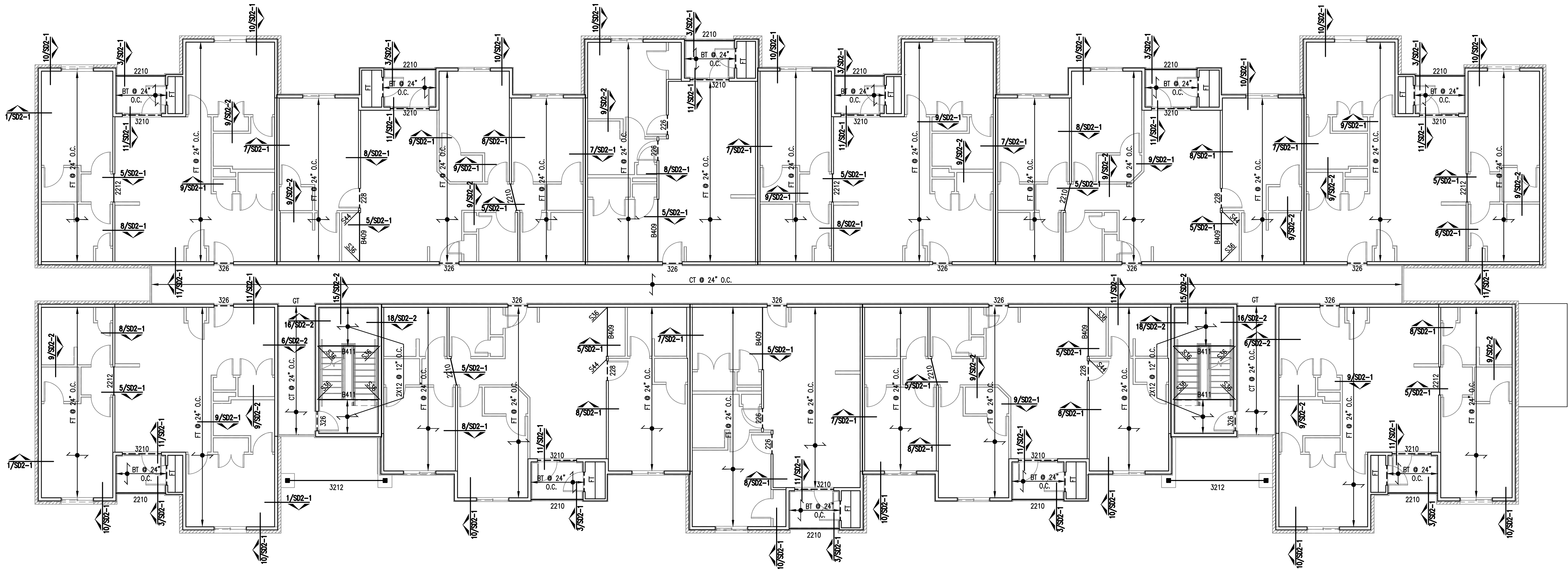


1 2ND FLOOR FRAMING PLAN – BUILDING TYPE III
SCALE: 1/8"=1'-0"

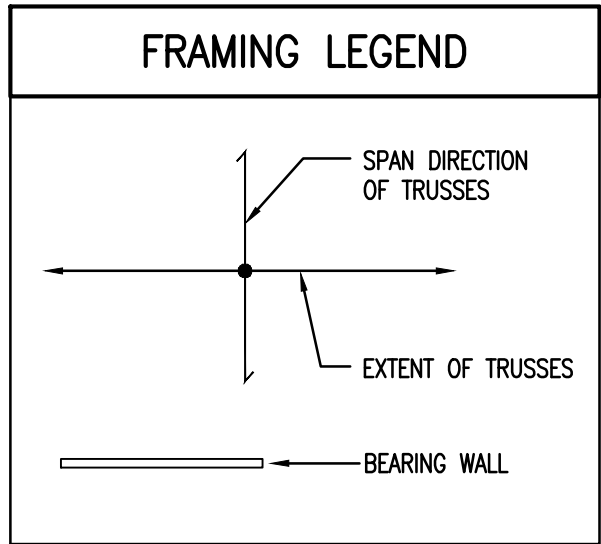


RE: SO-1, SO-3 & SO-5
FOR FRAMING NOTES
AND SCHEDULE

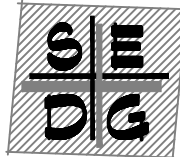
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2 3RD FLOOR FRAMING PLAN - BUILDING TYPE III
SCALE: 1/8"=1'-0"



RE: S0-1, S0-3 & S0-5
FOR FRAMING NOTES
AND SCHEDULE



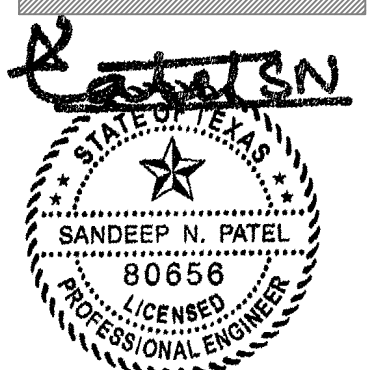
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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]
Sheet Title: 3RD FLOOR FRAMING PLAN - BUILDING TYPE III

Rev.	Description	Date

Drawn By: HT
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Drawing Scale: As Noted
Project No. 136-091

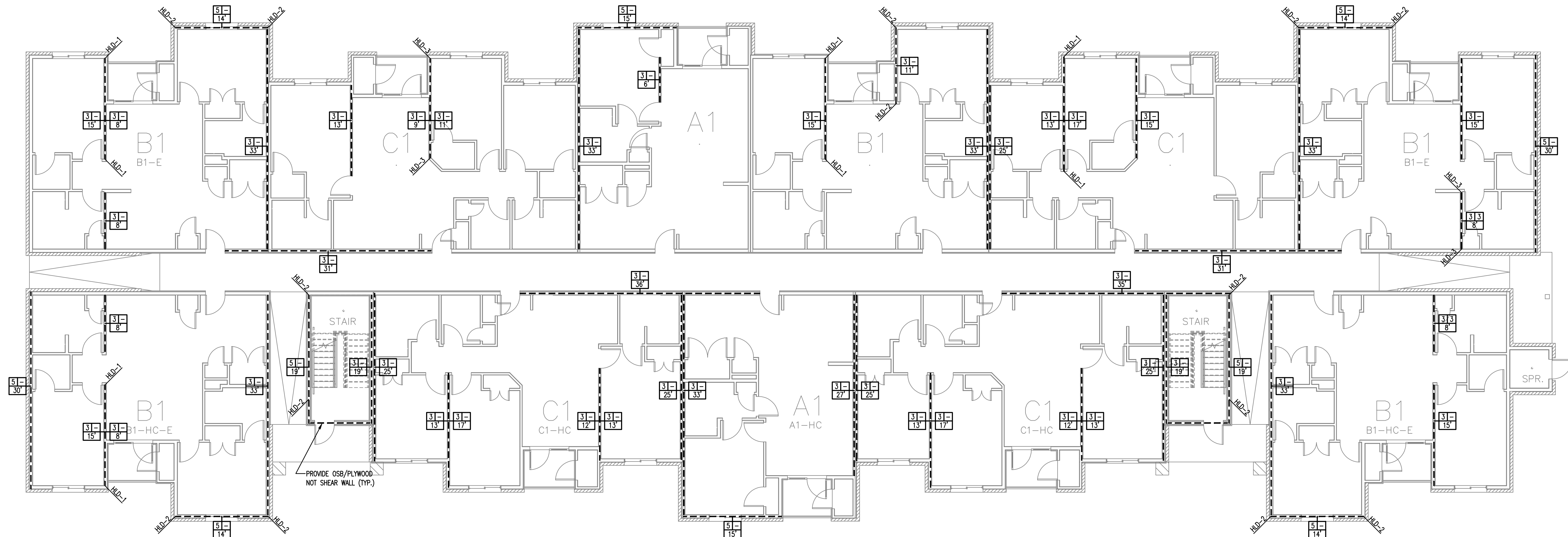
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Coordination
CD 95%
CD 100%
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Construction



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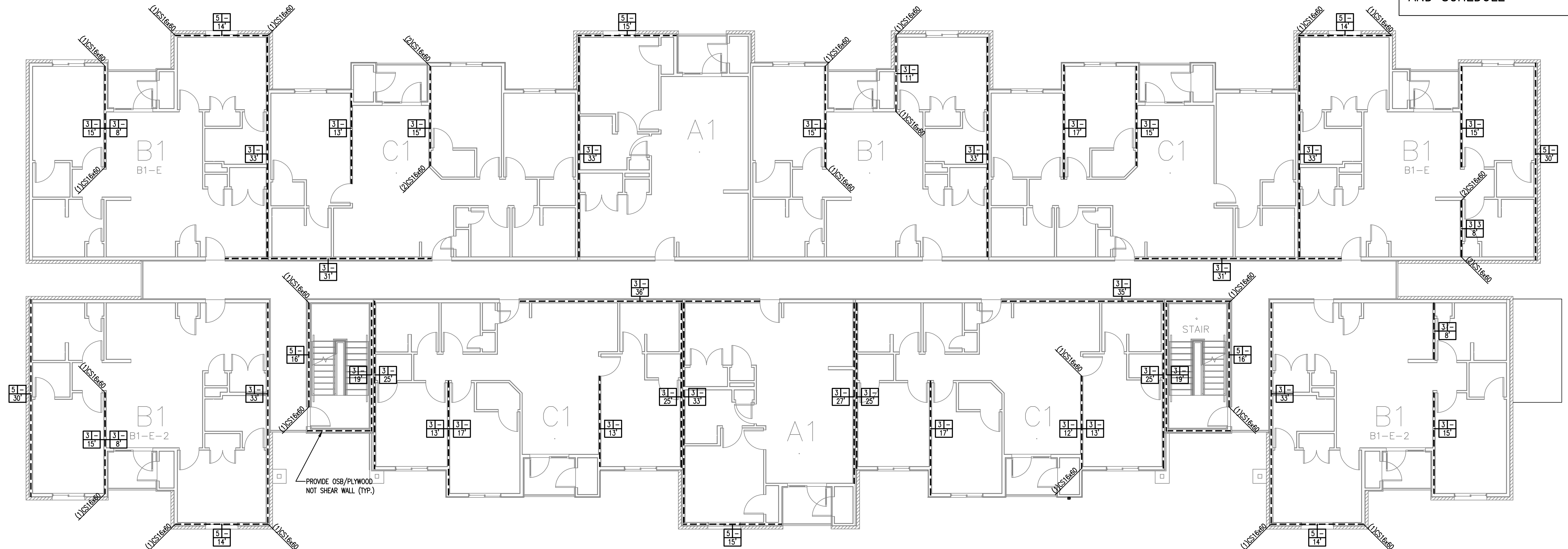
SHEET NO.
S2-3A
PLAN

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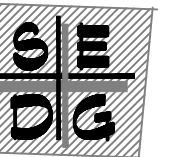


1 1ST FLOOR SHEARWALL PLAN - BUILDING TYPE I
SCALE: 1/8"=1'-0"

RE: S0-1, S0-3 & S0-5
FOR SHEARWALL NOTES
AND SCHEDULE



2 2ND FLOOR SHEARWALL PLAN - BUILDING TYPE I
SCALE: 1/8"=1'-0"



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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]
1ST AND 2ND FLOOR SHEARWALL PLAN
BUILDING TYPE I

Sheet Title:

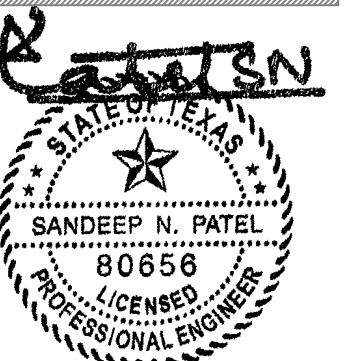
Date

Description

Rev.

Drawn By: HT
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Project No. 136-091

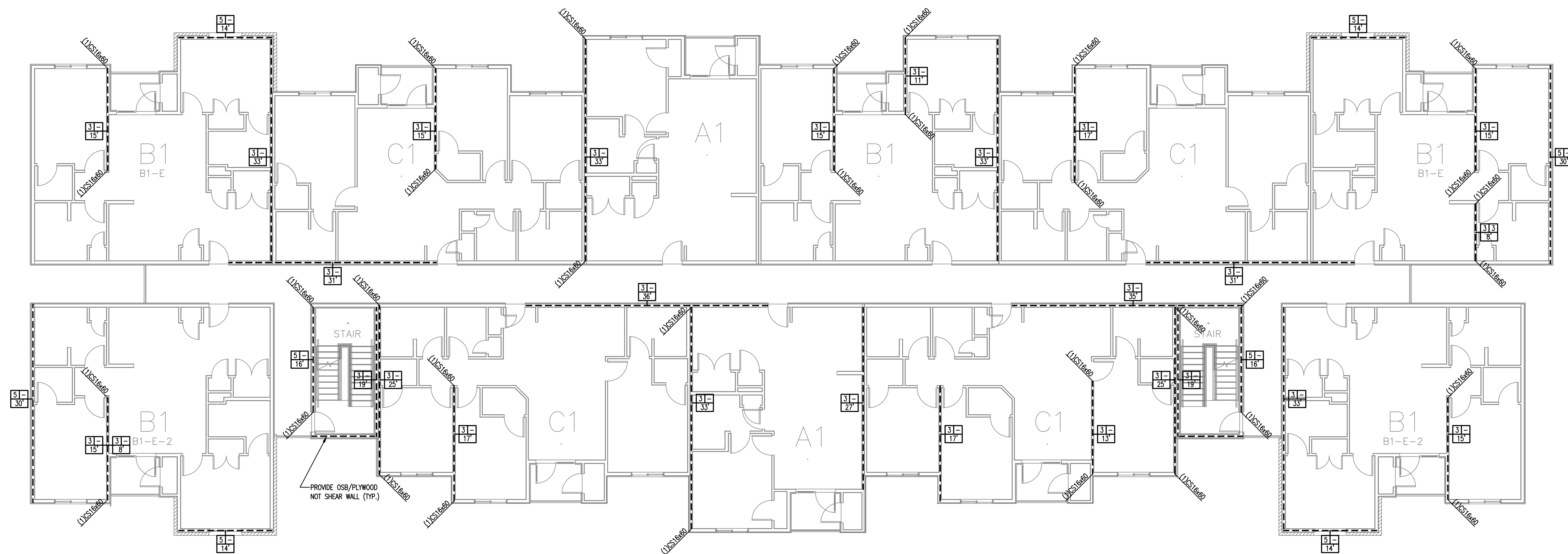
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SHEET NO.

S3-1
PLAN



1 3RD FLOOR SHEARWALL PLAN – BUILDING TYPE I
SCALE: 1/8"=1'-0"

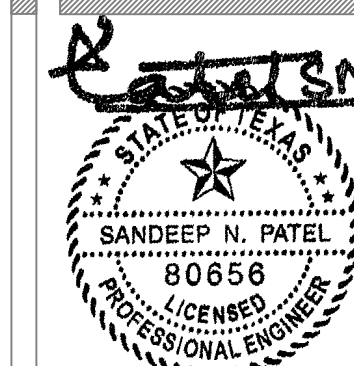
RE: S0-1, S0-3 & S0-5
FOR SHEARWALL NOTES
AND SCHEDULE

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

3RD FLOOR SHEARWALL PLAN
BUILDING TYPE I

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<input type="checkbox"/> Permit	_____
<input type="checkbox"/> Construction	_____



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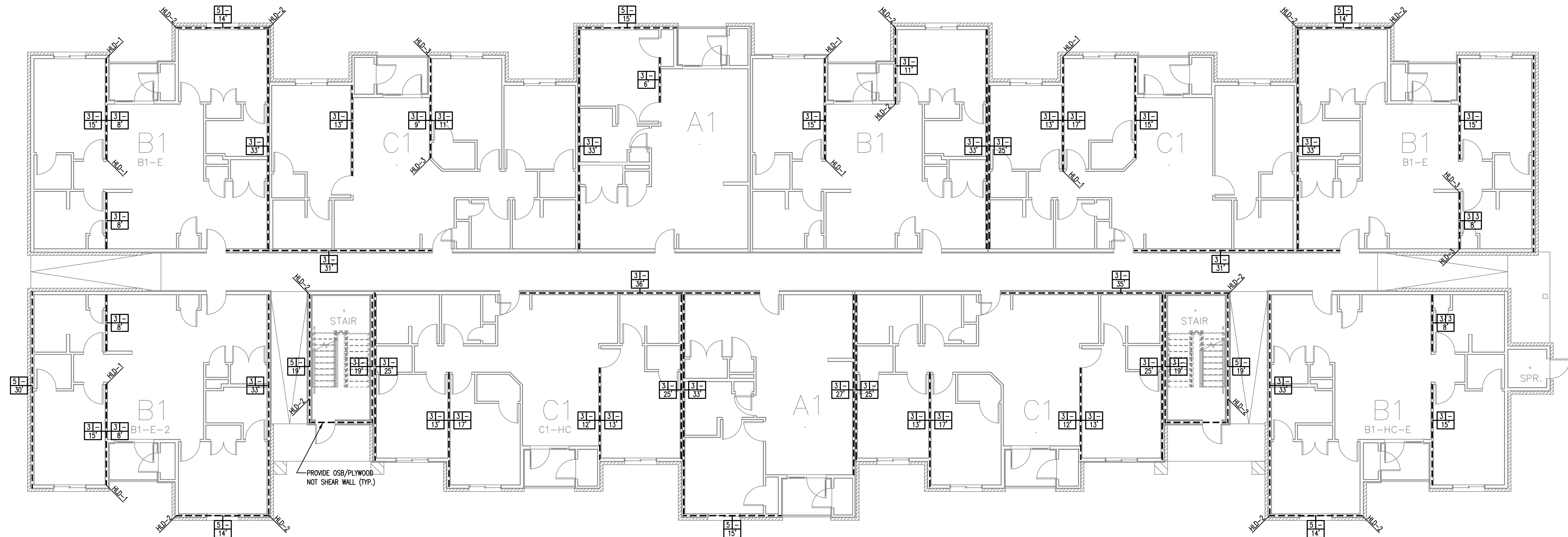
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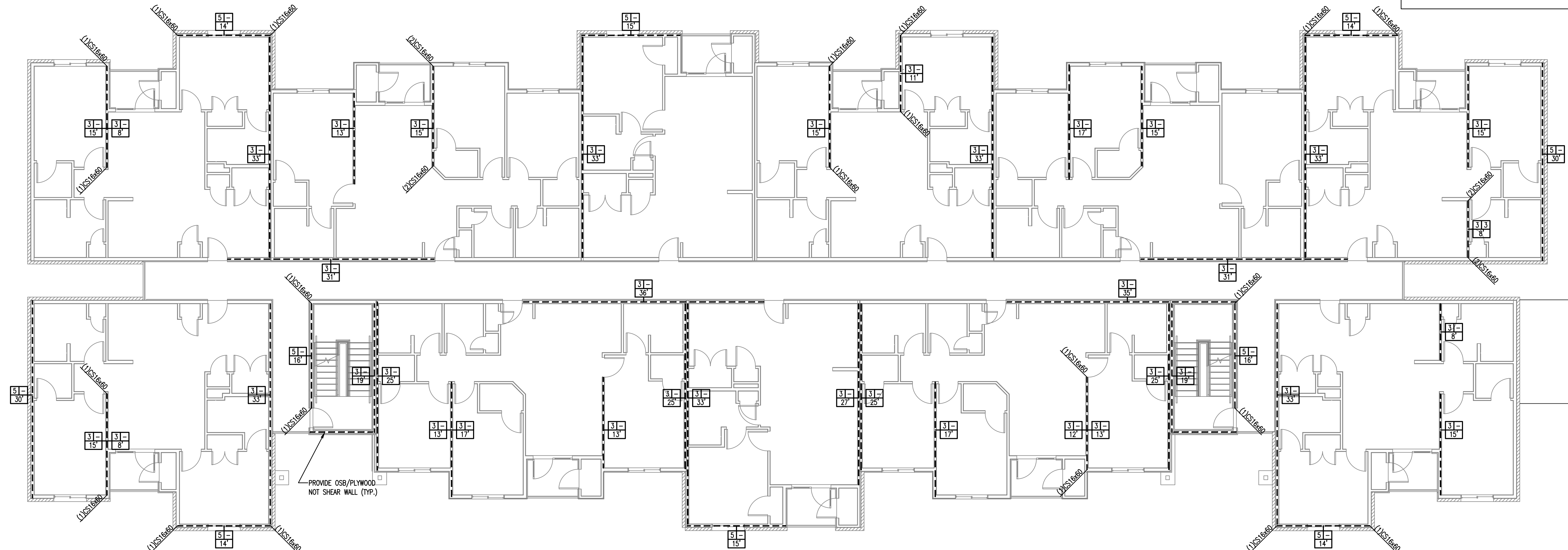
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SHEET NO.
S3-1A
PLAN

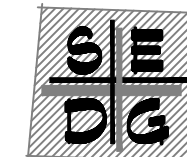
G:\S3-36 - KELLY GROSSMAN\136-091 Moonlight Ranch\Structural Drawings\Working Drawings\S3-2.dwg Plotted: June 10, 2019 - 11:03 AM by Hao Tran



1 1ST FLOOR SHEARWALL PLAN - BUILDING TYPE II
SCALE: 1/8"=1'-0"



2 2ND FLOOR SHEARWALL PLAN - BUILDING TYPE II
SCALE: 1/8"=1'-0"



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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]
1ST AND 2ND FLOOR SHEARWALL PLAN
BUILDING TYPE II

Sheet Title:

Date

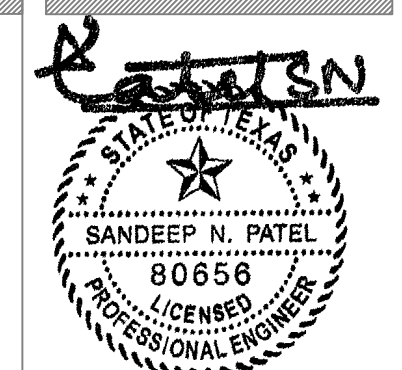
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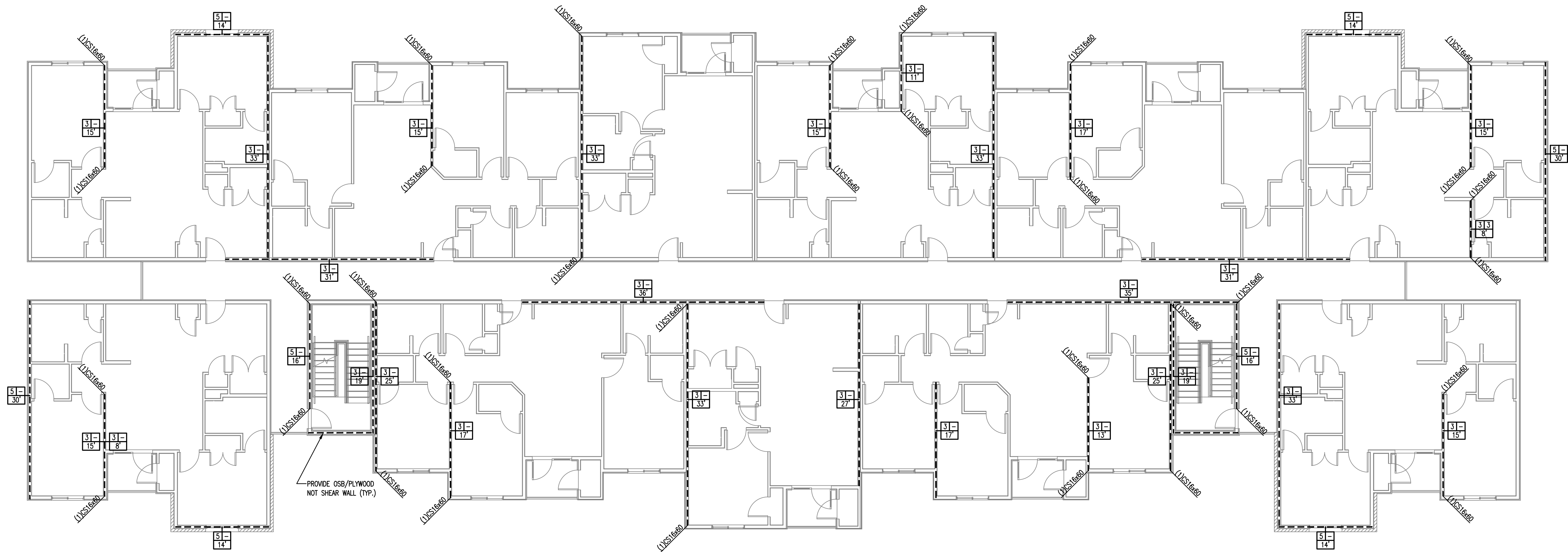
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S3-2

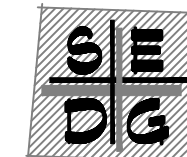
PLAN

G:\\$136 - KELLY GROSSMAN\136-091 Moonlight Ranch\Structural Drawings\Working Drawings\S3-2A.dwg Plotted: June 10, 2019 - 11:03 AM by Hao Tran



1 3RD FLOOR SHEARWALL PLAN - BUILDING TYPE II
SCALE: 1/8"=1'-0"

RE: S0-1, S0-3 & S0-5
FOR SHEARWALL NOTES
AND SCHEDULE



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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]
3RD FLOOR SHEARWALL PLAN
BUILDING TYPE II

Sheet Title:

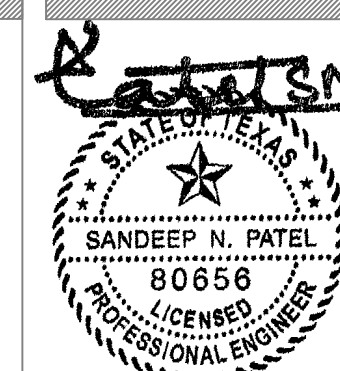
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Description

Rev.

Drawn By: HT
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Project No. 136-091

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☐ CD 100%
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☐ Bidding
☐ Permit
☐ Construction



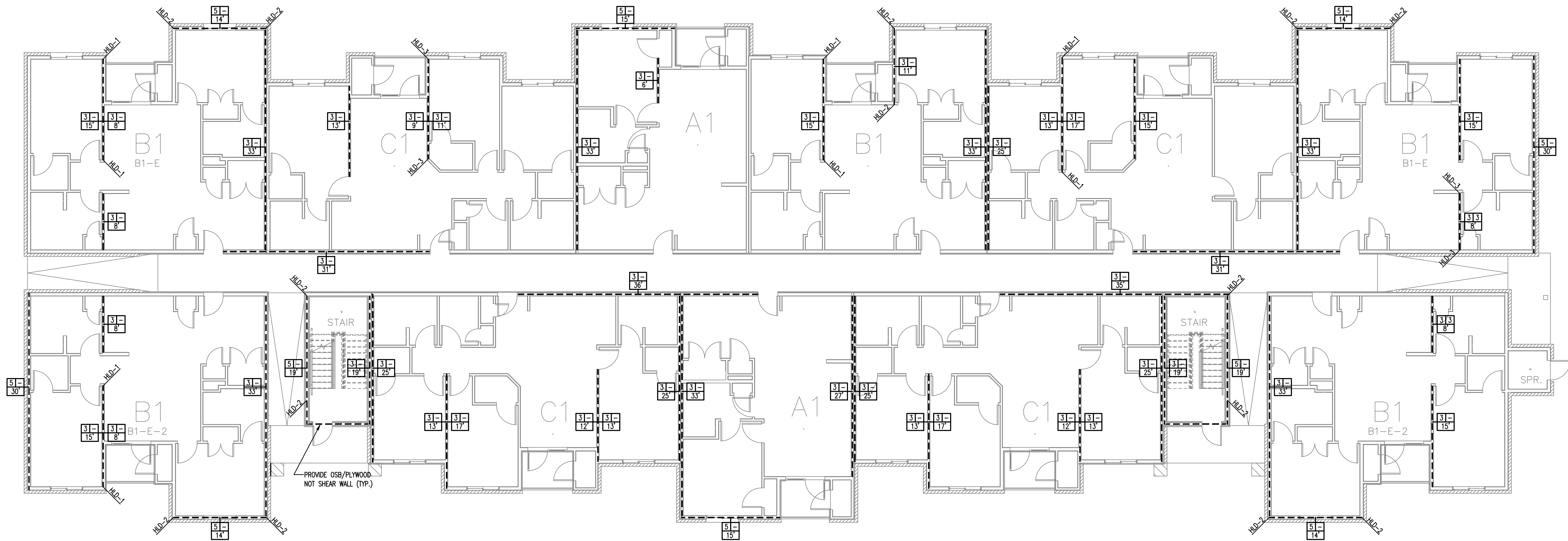
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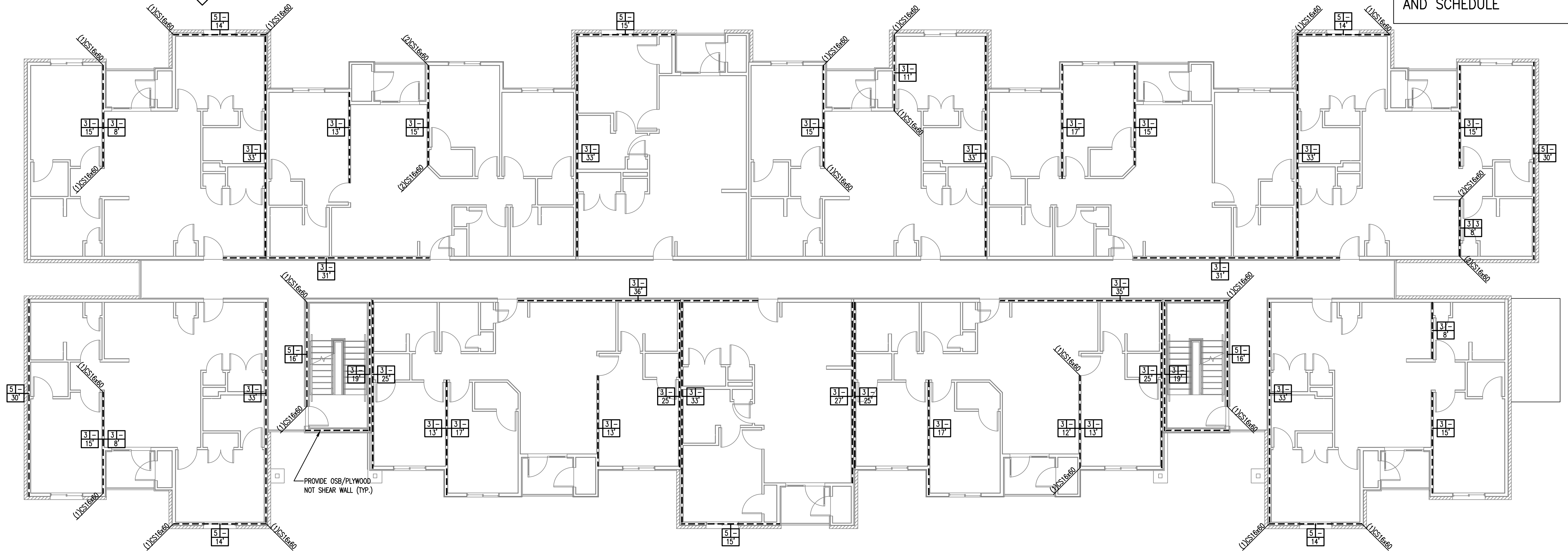
SHEET NO.

S3-2A
PLAN

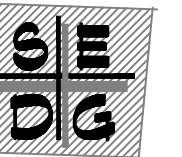
G:\S3-36 - KELLY GROSSMAN\136-091 Moonlight Ranch\Structural Drawings\Working Drawings\S3-3.dwg Plotted: June 10, 2019 - 11:03 AM by Hao Tran



1 1ST FLOOR SHEARWALL PLAN - BUILDING TYPE III
SCALE: 1/8"=1'-0"



2 2ND FLOOR SHEARWALL PLAN - BUILDING TYPE III
SCALE: 1/8"=1'-0"



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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]
1ST AND 2ND FLOOR SHEARWALL PLAN
BUILDING TYPE III

Sheet Title:

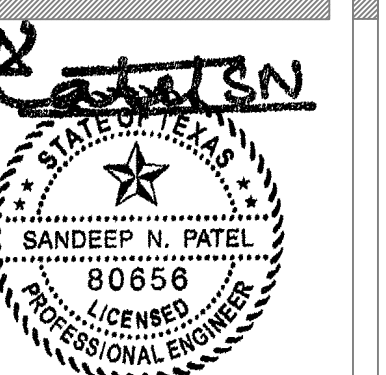
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Description

Rev.

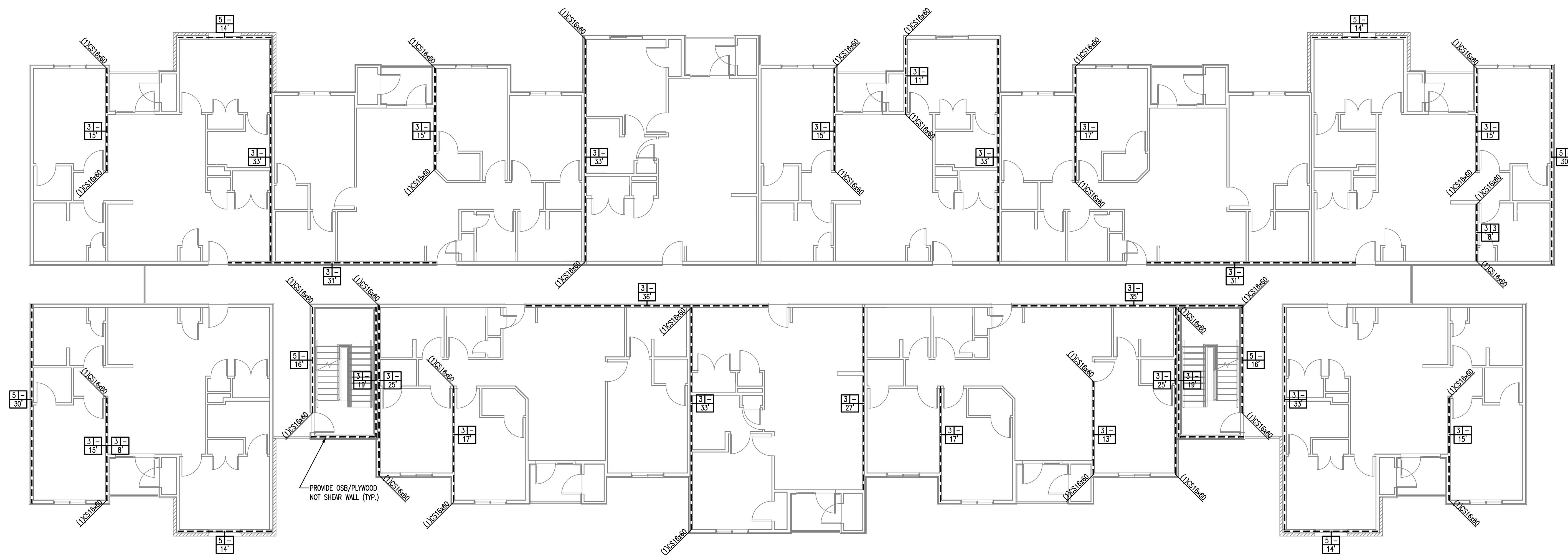
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Project No. 136-091

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SHEET NO.
S3-3
PLAN



1 3RD FLOOR SHEARWALL PLAN – BUILDING TYPE III
SCALE: 1/8"=1'-0"

RE: S0-1, S0-3 & S0-5
FOR SHEARWALL NOTES
AND SCHEDULE

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

3RD FLOOR SHEARWALL PLAN
BUILDING TYPE III

set Title:

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Drawn By:	Checked By:
HT	DVH/ZA
Drawing Scale:	Project No.
As Noted	136-091

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<input type="checkbox"/> Pricing	_____
<input type="checkbox"/> Bidding	_____
<input type="checkbox"/> Permit	_____
<input type="checkbox"/> Construction	_____

Sandeep N. Patel
STATE OF TEXAS
★
SANDEEP N. PATEL
80656
LICENSED
PROFESSIONAL ENGINEER

06/10/2019
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SHEET NO.
S3-3A
PLAN

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]
Sheet Title: **ROOF FRAMING PLAN - BUILDING TYPE I**

Sheet Title:

Date _____


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Drawing Scale:	Project No.
As Noted	136-091

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<input type="checkbox"/> Bidding	_____
<input type="checkbox"/> Permit	_____
<input type="checkbox"/> Construction	_____

Patel SN



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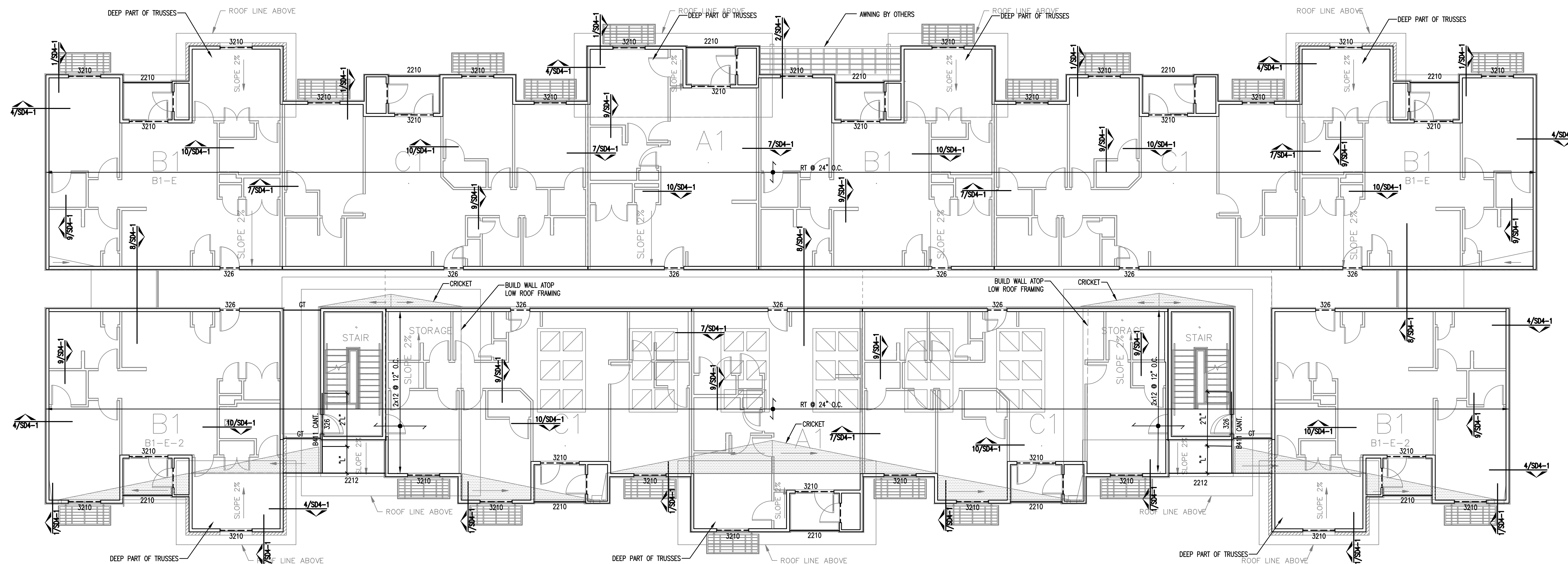
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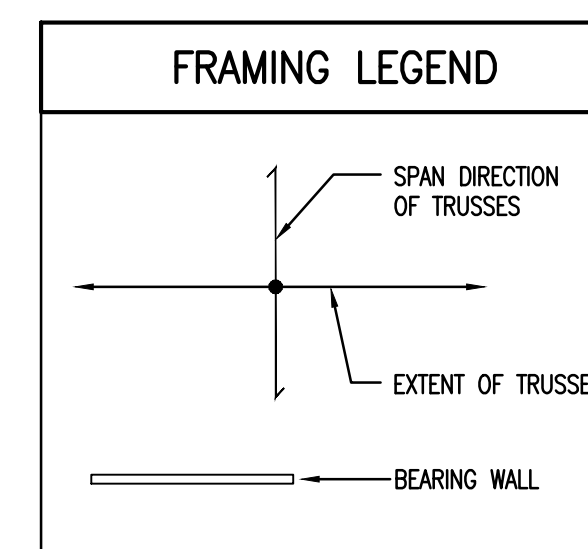
S4-1

PLAN



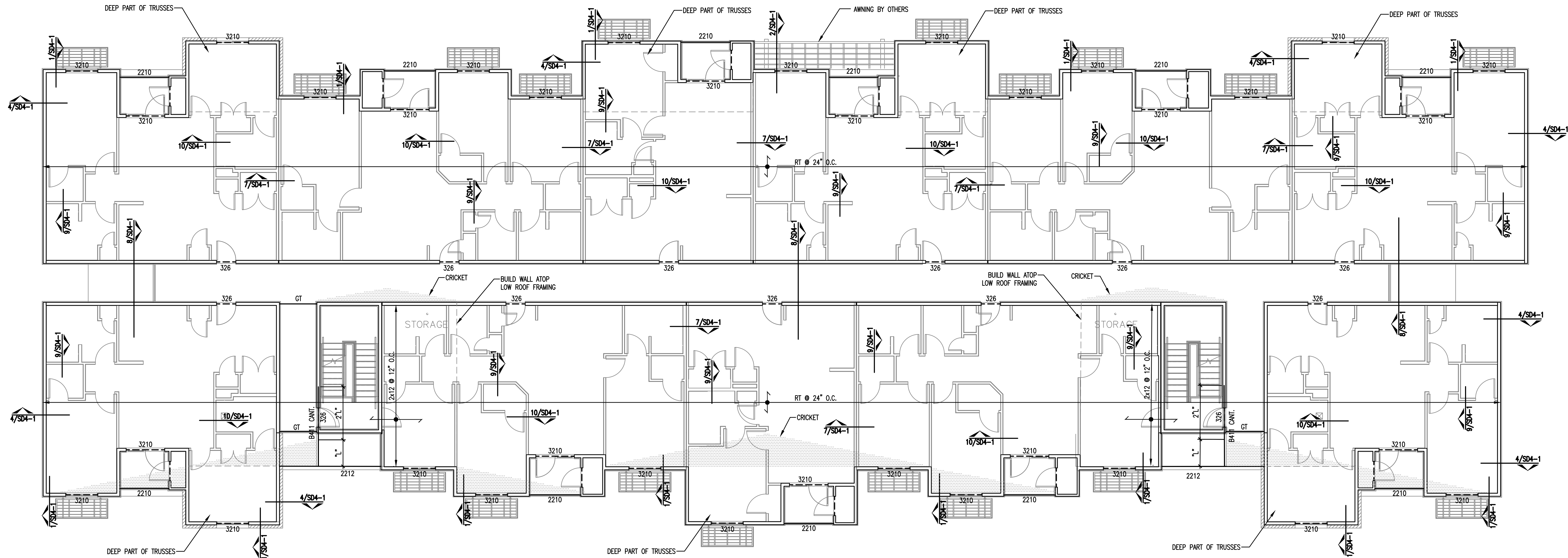
1 ROOF FRAMING PLAN – BLDG TYPE I
SCALE: 1/8"=1'-0"

SCALE: 1/8"=1'-0"



RE: S0-1, S0-3 & S0-5
FOR FRAMING NOTES
AND SCHEDULE

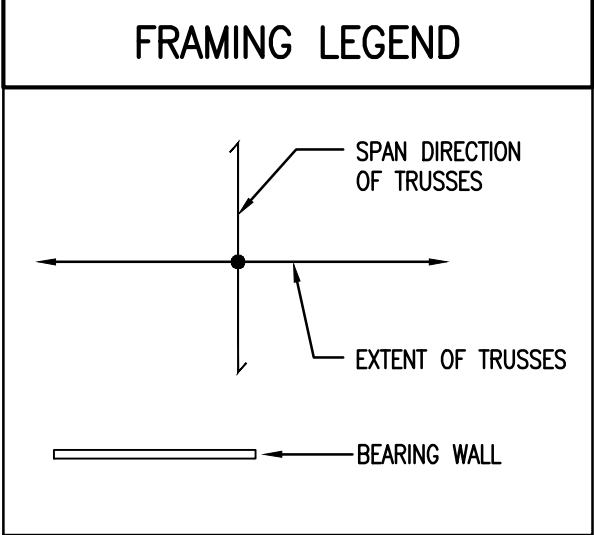
G:\\$136 - KELLY GROSSMAN\136-091 Moonlight Ranch\Structural Drawings\Working Drawings\S4-2.dwg Plotted: June 10, 2019 - 11:03 AM by Hao Tran



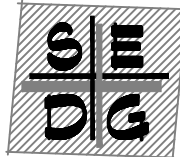
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ROOF FRAMING PLAN - BUILDING TYPE II

SCALE: 1/8"=1'-0"



RE: S0-1, S0-3 & S0-5
FOR FRAMING NOTES
AND SCHEDULE



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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

Sheet Title: ROOF FRAMING PLAN - BUILDING TYPE II

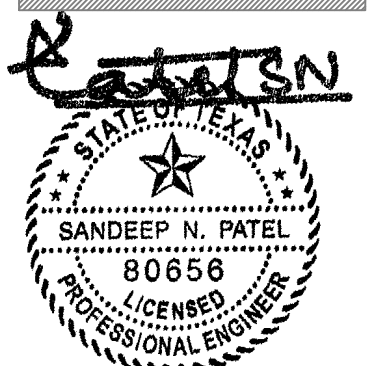
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Description

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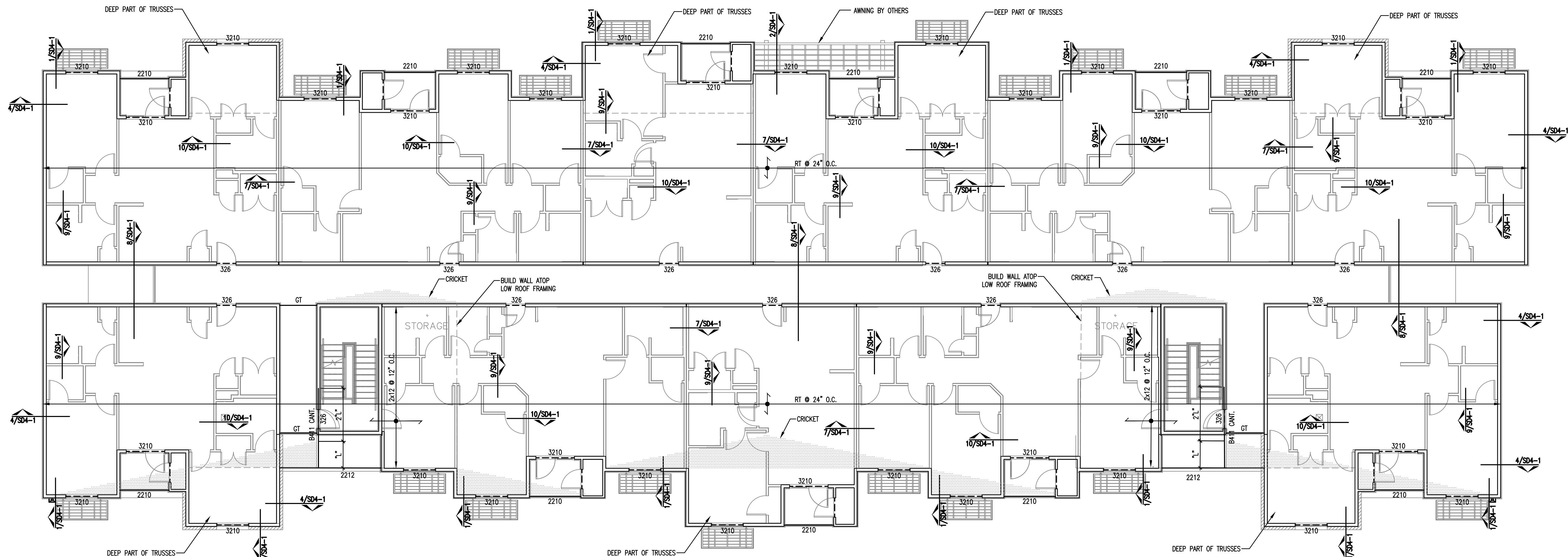
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SHEET NO.

S4-2

PLAN

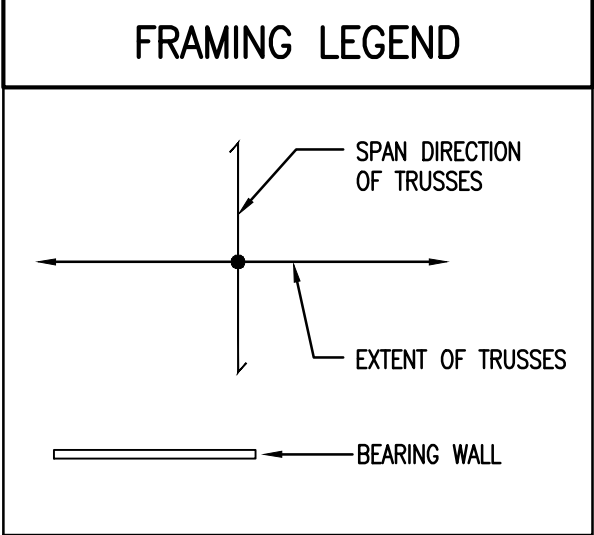
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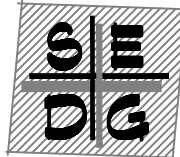
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ROOF FRAMING PLAN - BUILDING TYPE III

SCALE: 1/8"=1'-0"



RE: S0-1, S0-3 & S0-5
FOR FRAMING NOTES
AND SCHEDULE



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AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

Sheet Title: ROOF FRAMING PLAN - BUILDING TYPE III

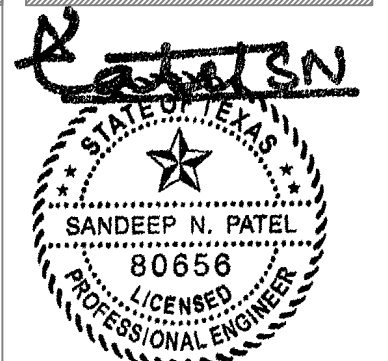
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Description

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Drawing Scale: As Noted
Project No. 136-091

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☐ CD 100%
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☐ Permit
☐ Construction



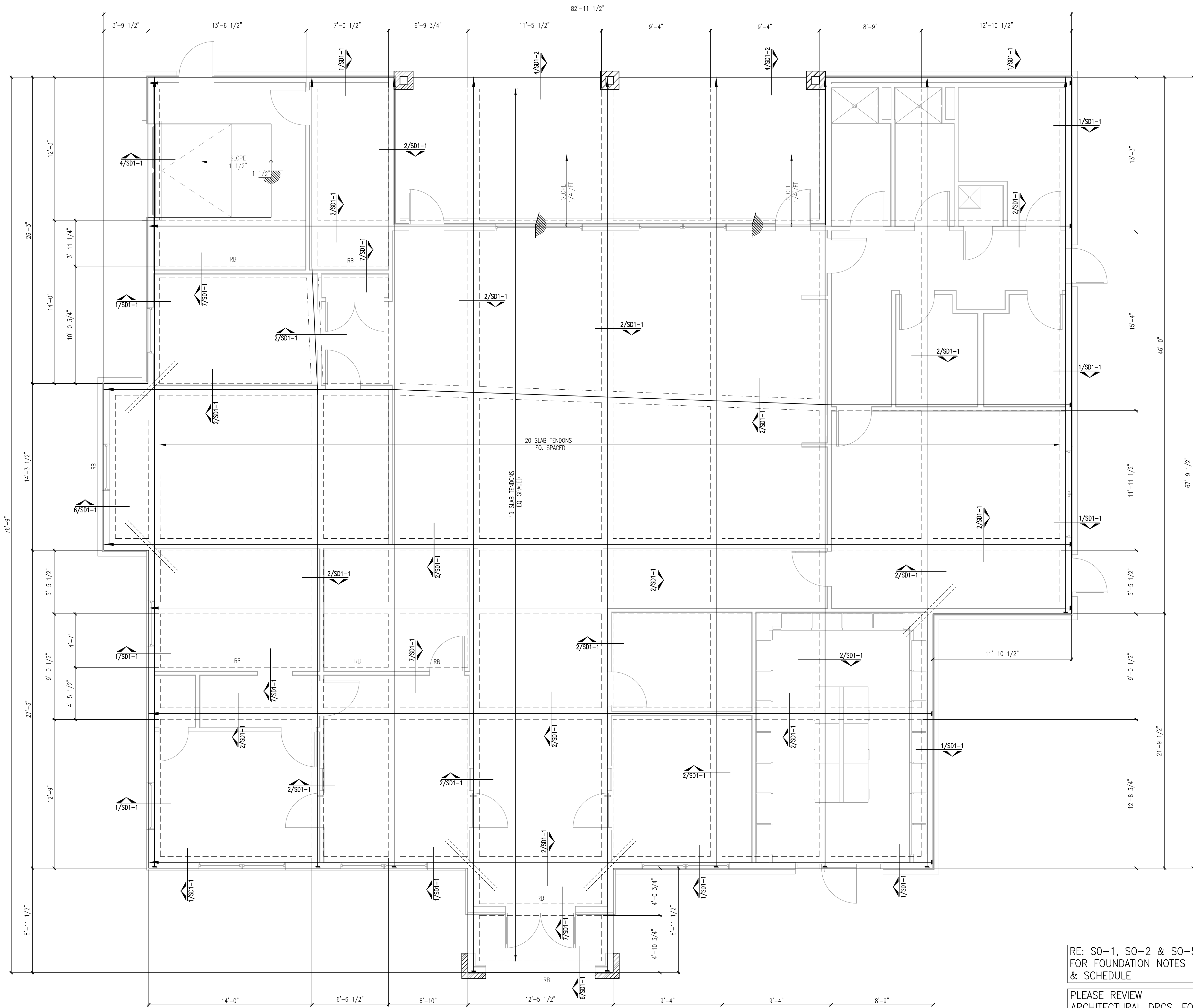
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S4-3

PLAN



1 FOUNDATION PLAN - CLUBHOUSE




SCALE: $1/4'' = 1' - 0''$

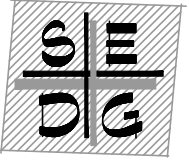
RE: SO-1, SO-2 & SO-5
FOR FOUNDATION NOTES
& SCHEDULE

PLEASE REVIEW
ARCHITECTURAL DRGS. FOR
DIMENSIONS, SLOPES,
DROPS & DEPRESSIONS.

SLAB NOTE

"1" THICK CONCRETE SLAB (UNLESS NOTED OTHERWISE)
ON MINIMUM TEN (10) MIL VAPOR BARRIER ON COMPACTED FILL.
SUBGRADE, THE VAPOR BARRIER SHALL CONFORM TO ASTM
E1745 CLASS A OR BETTER AND SHALL HAVE A MAXIMUM WATER
VAPOR PERMEANCE OF 0.01 PERMS WHEN TESTED IN
ACCORDANCE WITH ASTM E96.
THE VAPOR BARRIER SHALL BE INSTALLED PER THE
MANUFACTURER'S RECOMMENDATIONS AND ASTM E1643,
STANDARD PRACTICE FOR INSTALLATION OF WATER VAPOR
BARRIERS USED IN CONTACT WITH EARTH OR GRANULAR FILL
UNDER CONCRETE SLABS. REFER TO GEOTECHNICAL REPORT FOR
SUBGRADE PREPARATION AND COMPACTION REQUIREMENTS.

GRADE BEAM SCHEDULE				
PLAN MARK	PLAN SYM.	BEAM WIDTH	BEAM DEPTH	NOTES
THICKENED SLAB		RE-PLAN	12"	2-#5 BOTT.
REINF. BEAM		12"	30"	2-#5 BOTT. TIES EACH TENDON
GRADE BEAM		12"	30"	1 TENDON



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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS

AUSTIN, TEXAS
Development Bv [IDG]

A Development By [LDG]
Architecture By [KELLY GROSSMAN]

Sheet Title:

FOUNDATION PLAN - CLUBHOUSE

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<input type="checkbox"/> CD 95%	_____
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<input type="checkbox"/> Pricing	_____
<input type="checkbox"/> Bidding	_____
<input type="checkbox"/> Permit	_____
<input type="checkbox"/> Construction	_____

Handwritten: K. N. Patel

STATE OF TEXAS
★
SANDEEP N. PATEL
80656
LICENSED
PROFESSIONAL ENGINEER

06/10/2019
Texas Registered Engineering Firm

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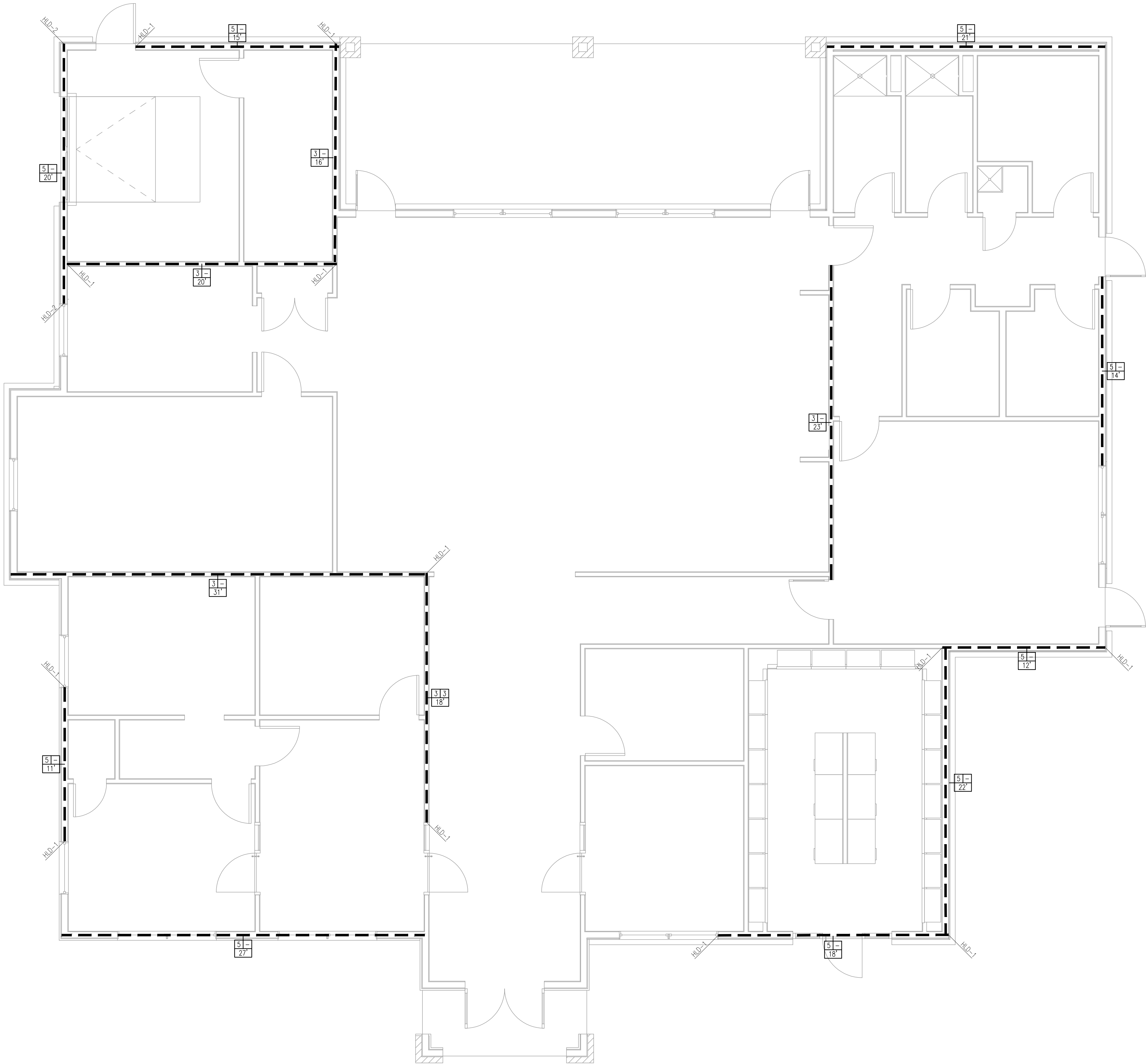
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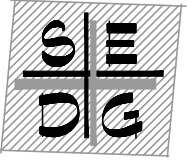
S5-1

PLAN



1 SHEARWALL PLAN - CLUBHOUSE
SCALE: 1/4"=1'-0"

RE: SO-1, SO-3 & SO-5
FOR SHEARWALL NOTES
AND SCHEDULE



**STERLING ENGINEERING
DESIGN GROUP**
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Suite # 201
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P:(281)583-7088 F:(281)583-5495

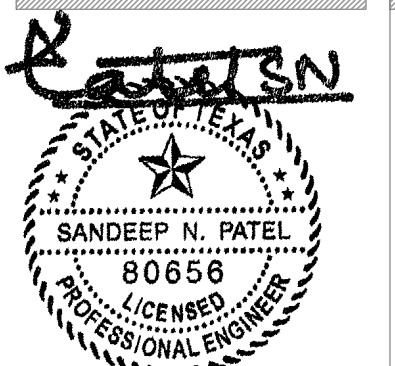
MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

at Title: **SHEARWALL PLAN - CLUBHOUSE**

[illegible]

Drawn By:	Checked By:
HT	DVH/ZA
Drawing Scale:	Project No.
As Noted	136-091

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<input type="checkbox"/> Coordination	_____
<input type="checkbox"/> CD 95%	_____
<input type="checkbox"/> CD 100%	_____
<input type="checkbox"/> Pricing	_____
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<input type="checkbox"/> Permit	_____
<input type="checkbox"/> Construction	_____



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SHEET NO.

S5-2

PLAN

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS

AUSTIN, TEXAS
A Development By [LDG]

A development by [LDO]
Architecture By [KELLY GROSSMAN]

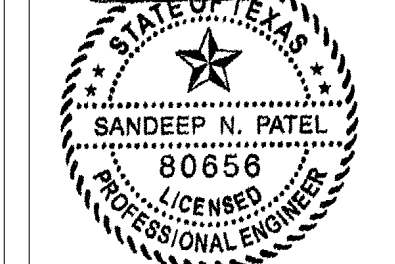
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Drawn By:	Checked By:
HT	DVH/ZA
Drawing Scale:	Project No.
As Noted	136-091

ISSUED FOR:	DATE:
<input type="checkbox"/> SD 30%	_____
<input type="checkbox"/> Coordination	_____
<input type="checkbox"/> CD 95%	_____
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<input type="checkbox"/> Construction	_____

Kathryn



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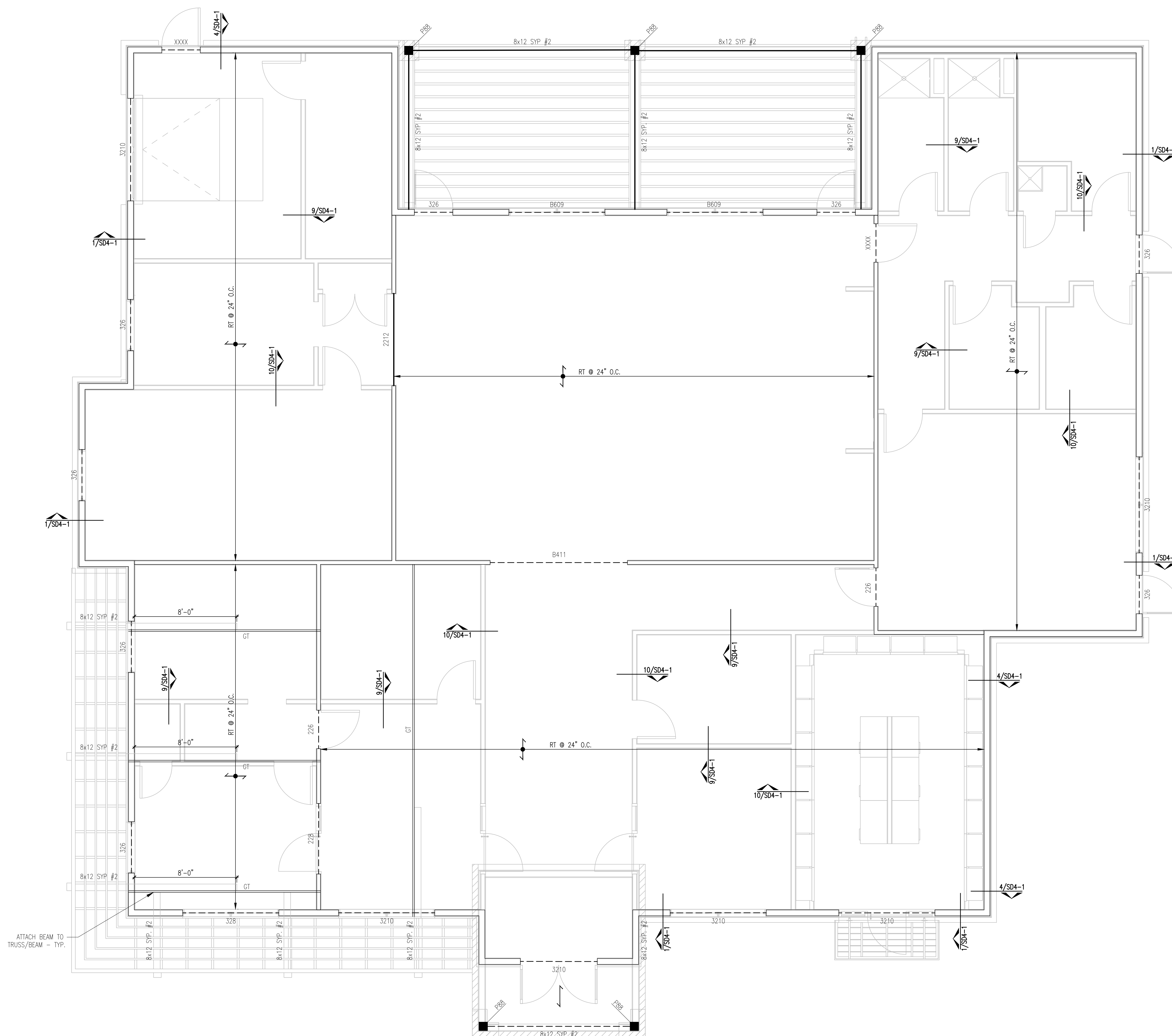
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S5-3

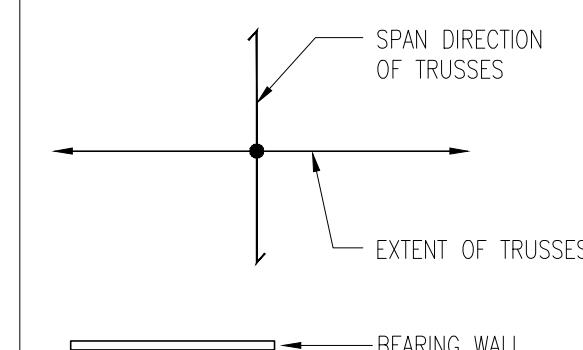
PLAN



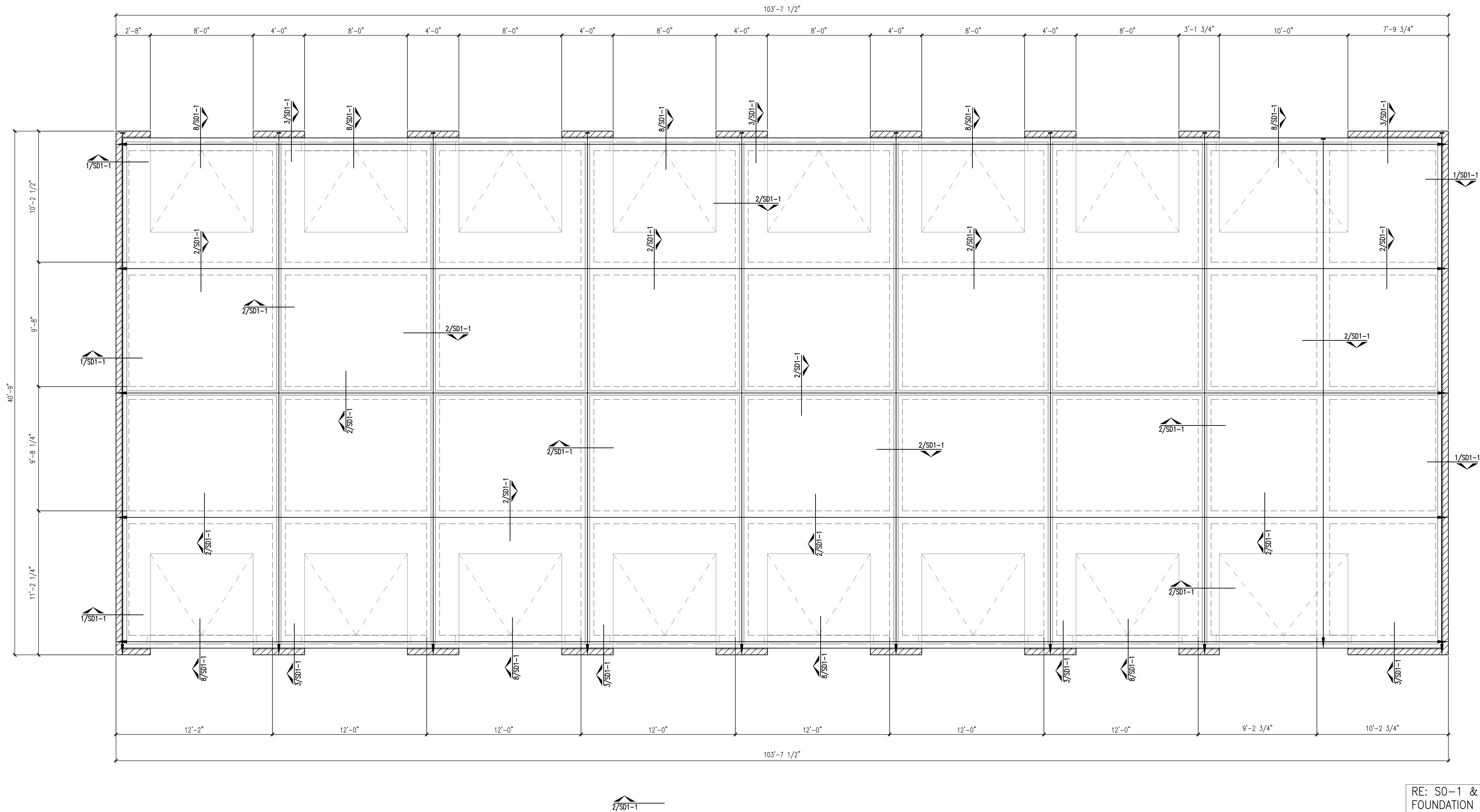
1 ROOF FRAMING PLAN - CLUBHOUSE
SCALE: 1/4"=1'-0"

RE: SO-1, SO-3 & SO-5
FOR FRAMING NOTES
AND SCHEDULE

FRAMING LEGEND



G:\\$136 - KELLY GROSSMAN\136-091 Moonlight Ranch\Structural Drawings\Working Drawings\556-1.dwg Plotted: June 10, 2019 - 11:03 AM by Hao Tran



1 FOUNDATION PLAN - GARAGE

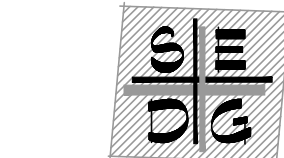
SCALE: 1/4"=1'-0"

RE: S0-1 & S0-2 FOR
FOUNDATION NOTES
& SCHEDULE

PLEASE REVIEW
ARCHITECTURAL DRGS.
FOR DIMENSIONS, SLOPES,
DROPS & DEPRESSIONS.

SLAB NOTE

"5." THICK CONCRETE SLAB (UNLESS NOTED OTHERWISE)
ON MINIMUM TEN (10) MIL VAPOR BARRIER ON COMPACTED
FILL SUBGRADE. THE VAPOR BARRIER SHALL CONFORM TO
ASTM E1745 CLASS A OR BETTER AND SHALL HAVE A
MAXIMUM WATER VAPOR PERMEANCE OF 0.01 PERMS
WHEN TESTED IN ACCORDANCE WITH ASTM E96.
THE VAPOR BARRIER SHALL BE INSTALLED PER THE
MANUFACTURER'S RECOMMENDATIONS AND ASTM E1643,
STANDARD PRACTICE FOR INSTALLATION OF WATER VAPOR
BARRIERS USED IN CONTACT WITH EARTH OR GRANULAR
FILL UNDER CONCRETE SLABS. REFER TO GEOTECHNICAL
REPORT FOR SUBGRADE PREPARATION AND COMPACTION
REQUIREMENTS.



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AUSTIN, TEXAS

A Development By [LDG]
Architecture By [KELLY GROSSMAN]

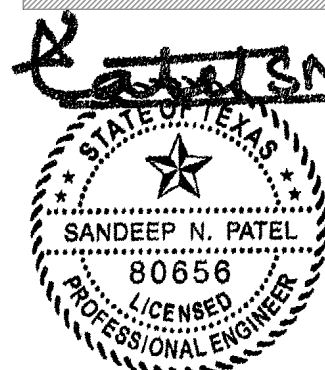
FOUNDATION PLAN - GARAGE

Sheet Title:

Rev.	Description	Date

Drawn By: HT
Checked By: DWH/ZA
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Project No. 136-091

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SHEET NO.

S6-1
PLAN

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

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<input type="checkbox"/> Bidding	_____
<input type="checkbox"/> Permit	_____
<input type="checkbox"/> Construction	_____

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SANDEEP N. PATEL
80656
LICENSED
PROFESSIONAL ENGINEER

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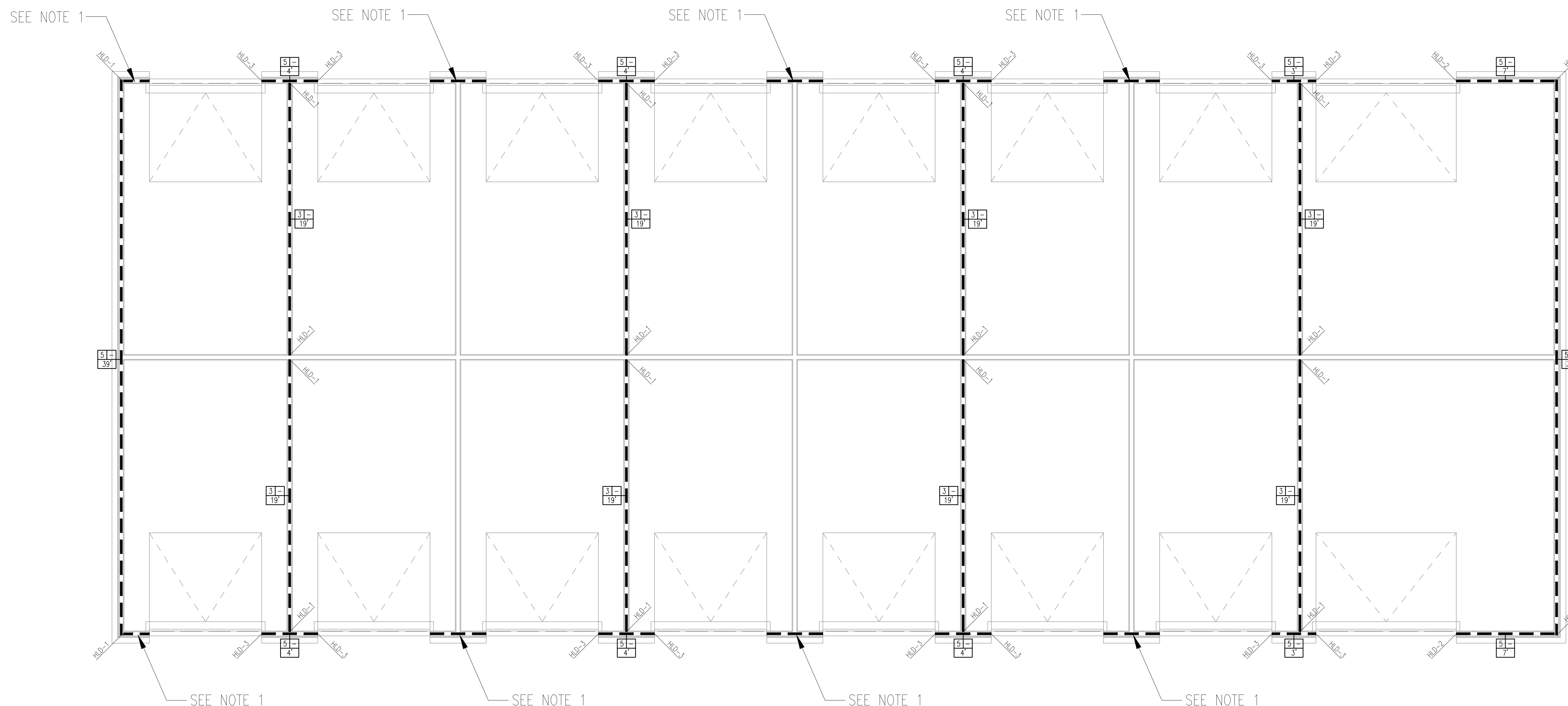
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S6-2

PLAN



NOTE 1:
SHEATH W/ OSB
NOT SHEAR WALL - TYP.

1 SHEARWALL PLAN - GARAGE
SCALE: 1/4"=1'-0"

SCALE: 1/4"=1'-0"

RE: SO-1, SO-3 & SO-5
FOR SHEARWALL NOTES
AND SCHEDULE

MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

set Title: **ROOF PLAN - GARAGE**


Sheet Title:

[illegible]

Drawn By:	Checked By:
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Drawing Scale:	Project No.
As Noted	136-091

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Sandeep N. Patel



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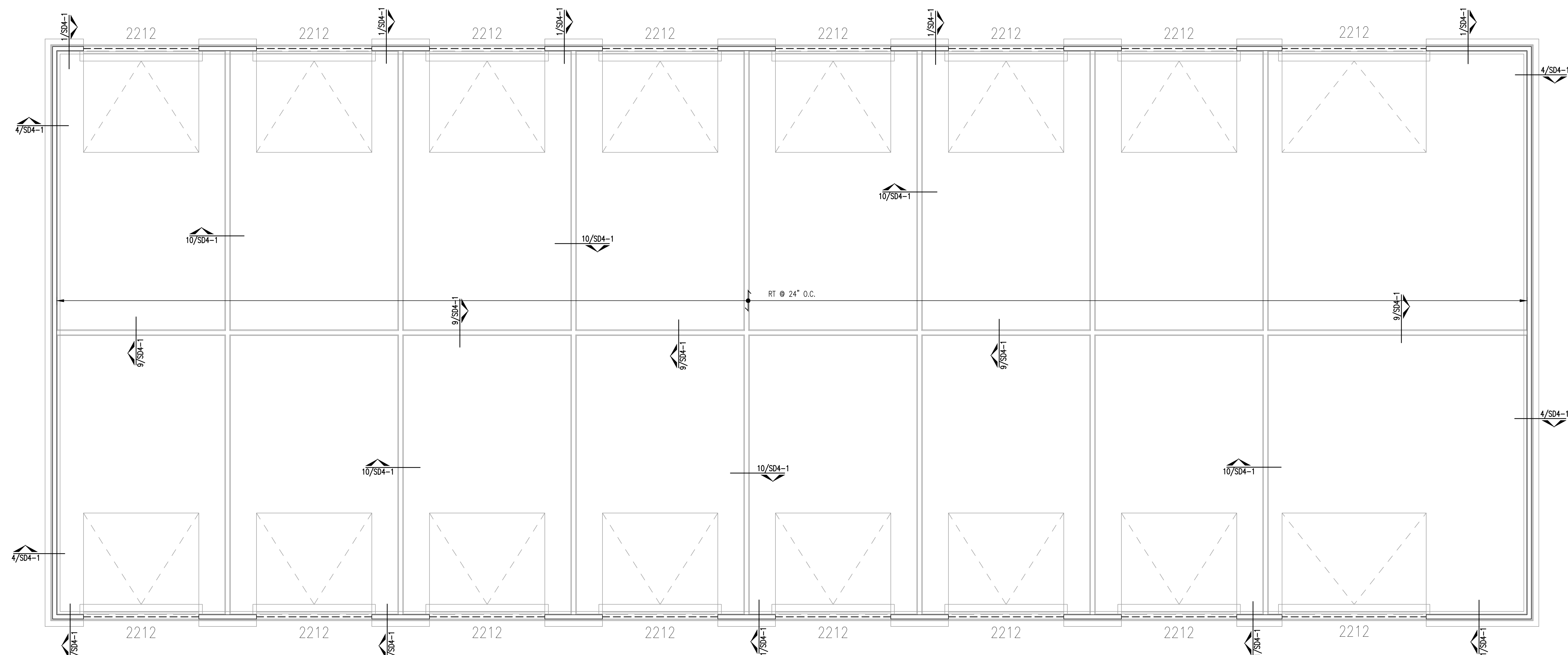
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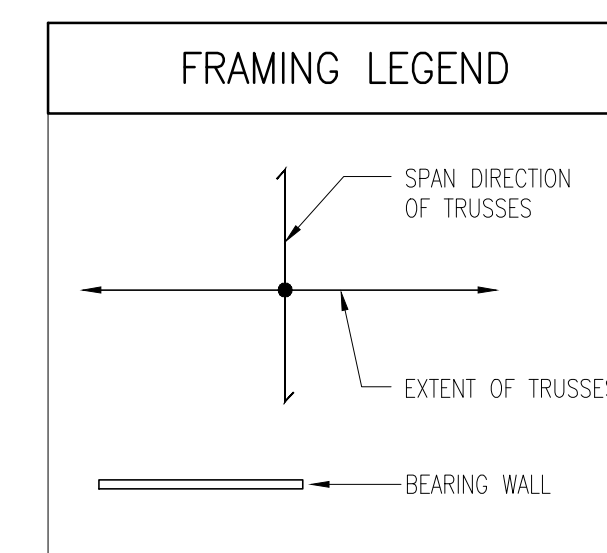
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SHEET NO.

S6-3

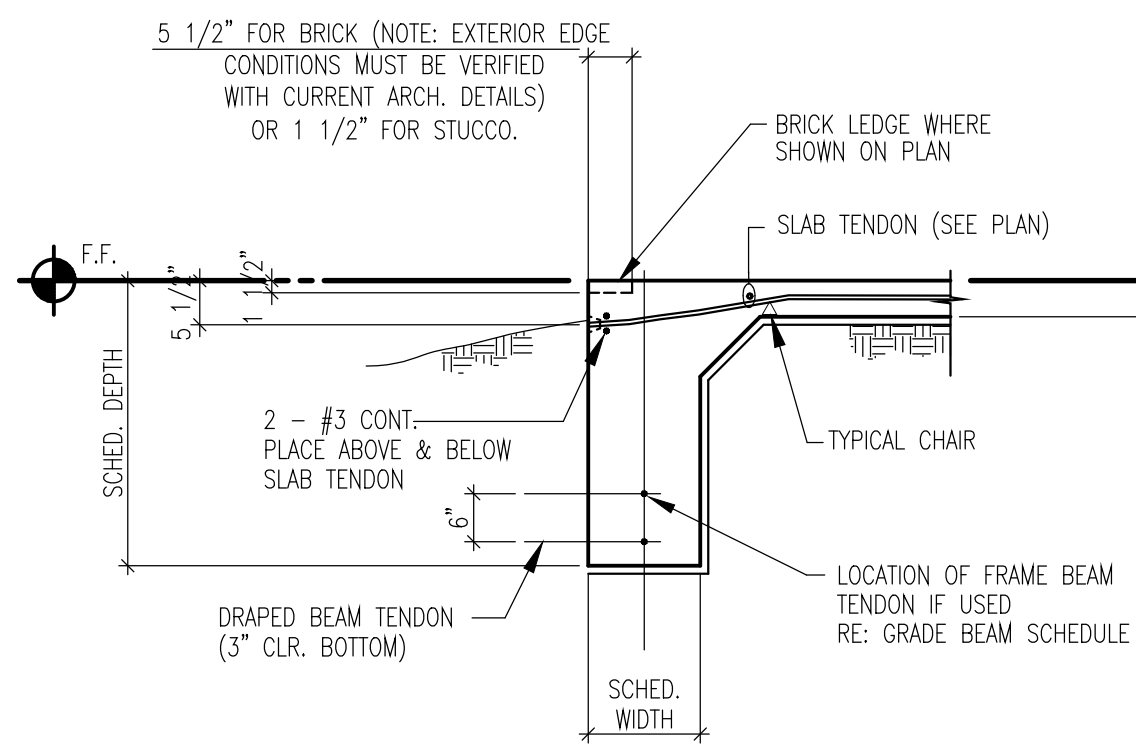


1 ROOF PLAN - GARAGE
SCALE: 1/4"=1'-0"

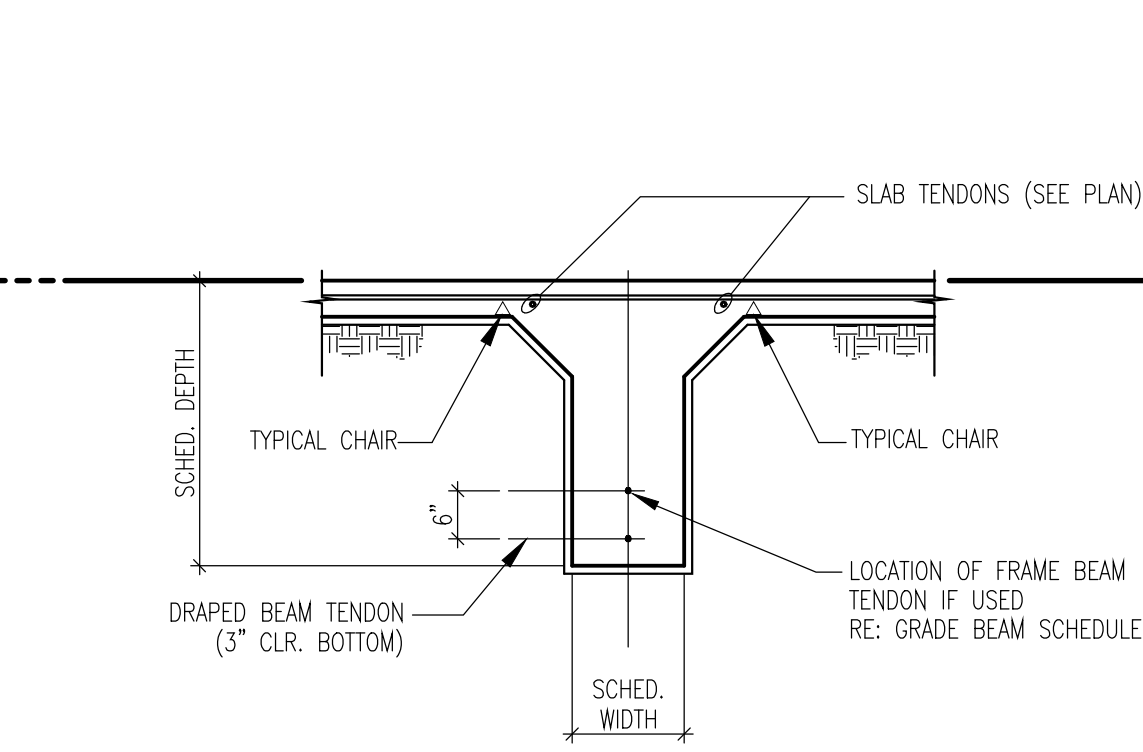


RE: SO-1, SO-3 & SO-5
FOR FRAMING NOTES
AND SCHEDULE

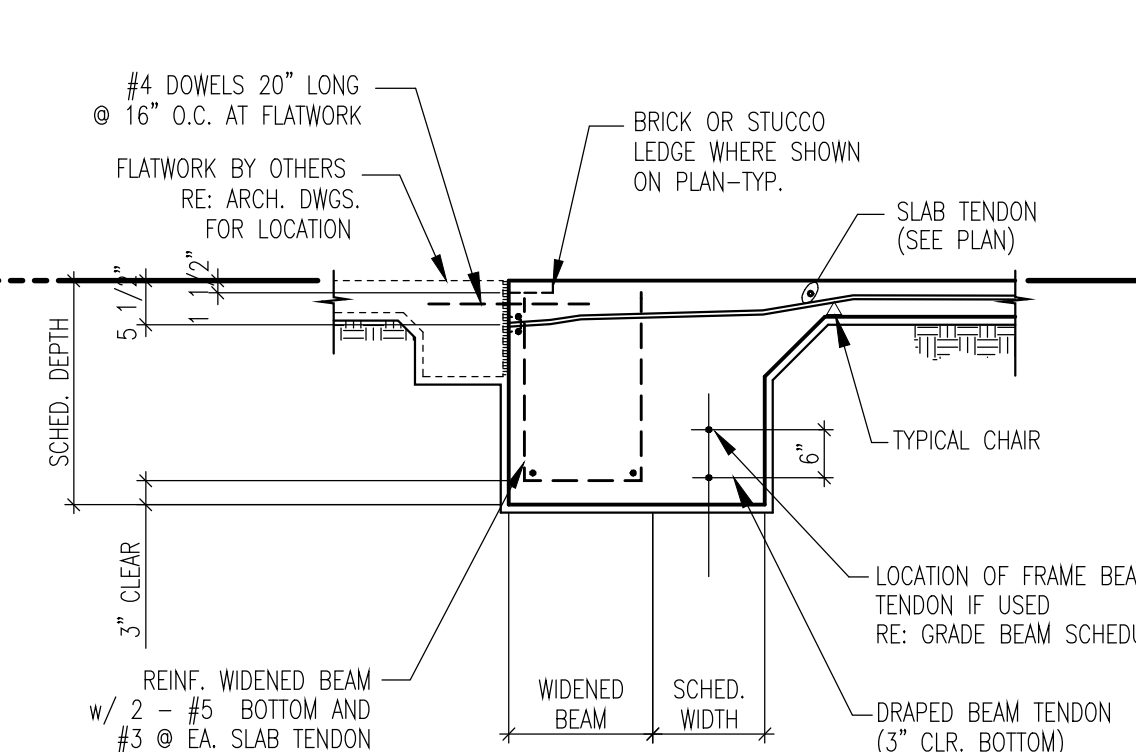
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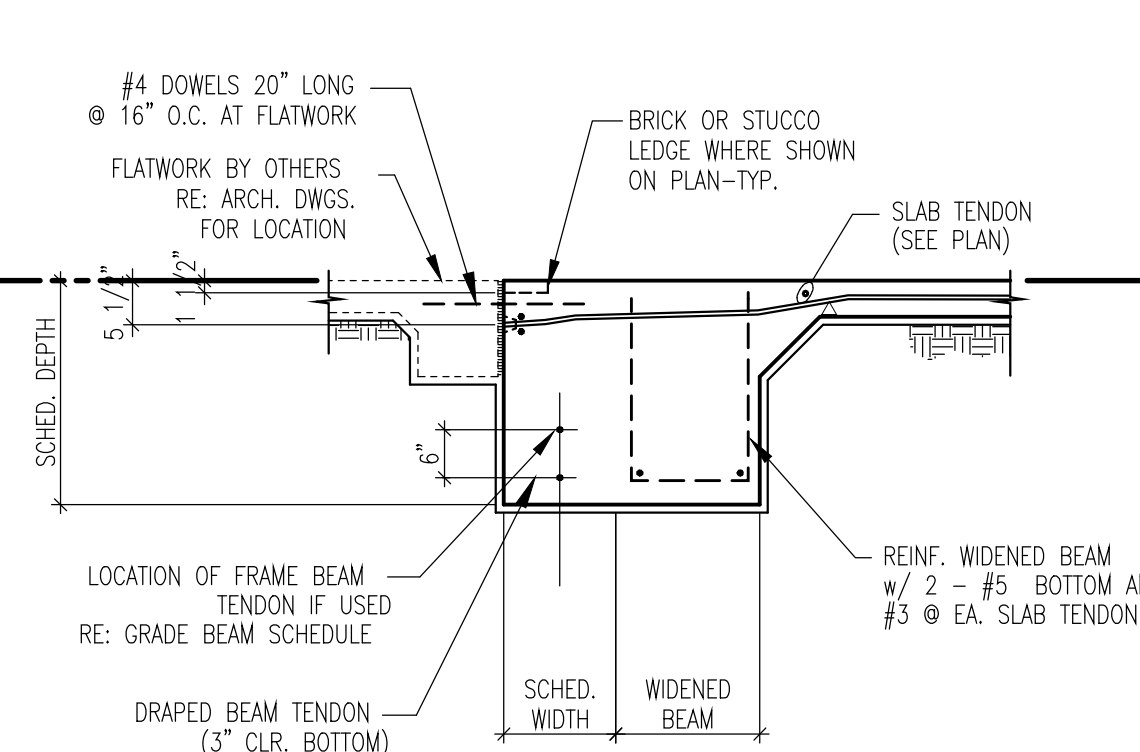
1 TYPICAL EXTERIOR GRADE BEAM
SCALE: 1/2"=1'-0"



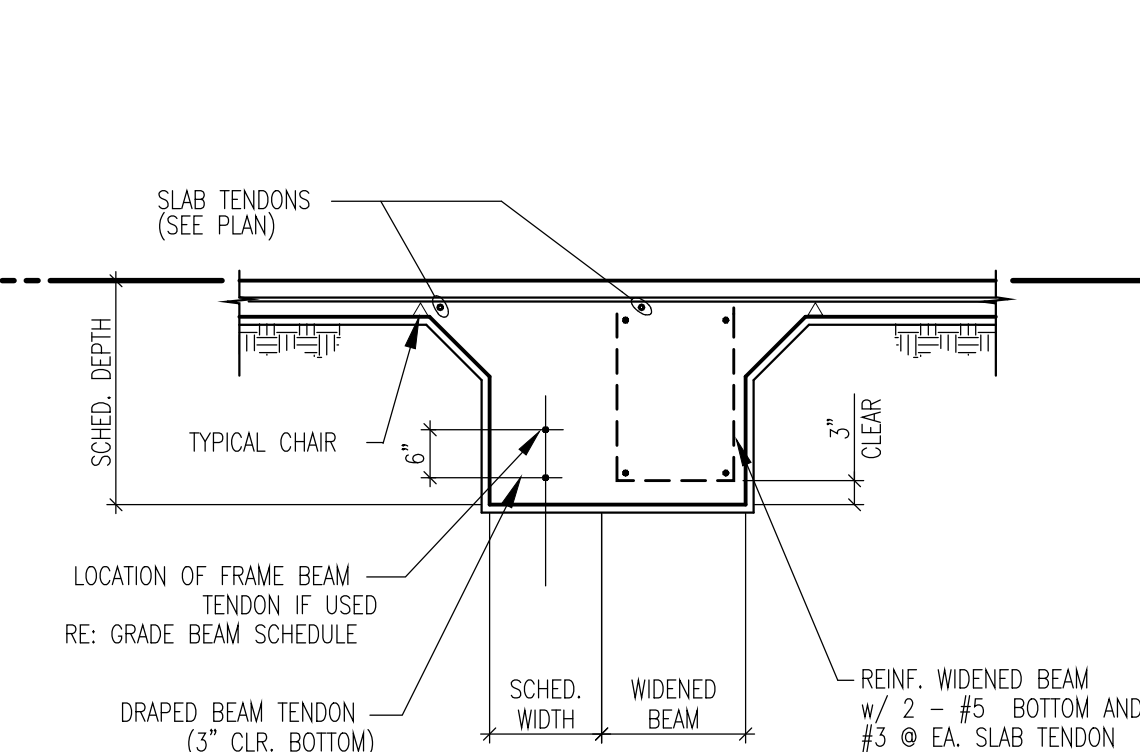
2 TYPICAL INTERIOR GRADE BEAM
SCALE: 1/2"=1'-0"



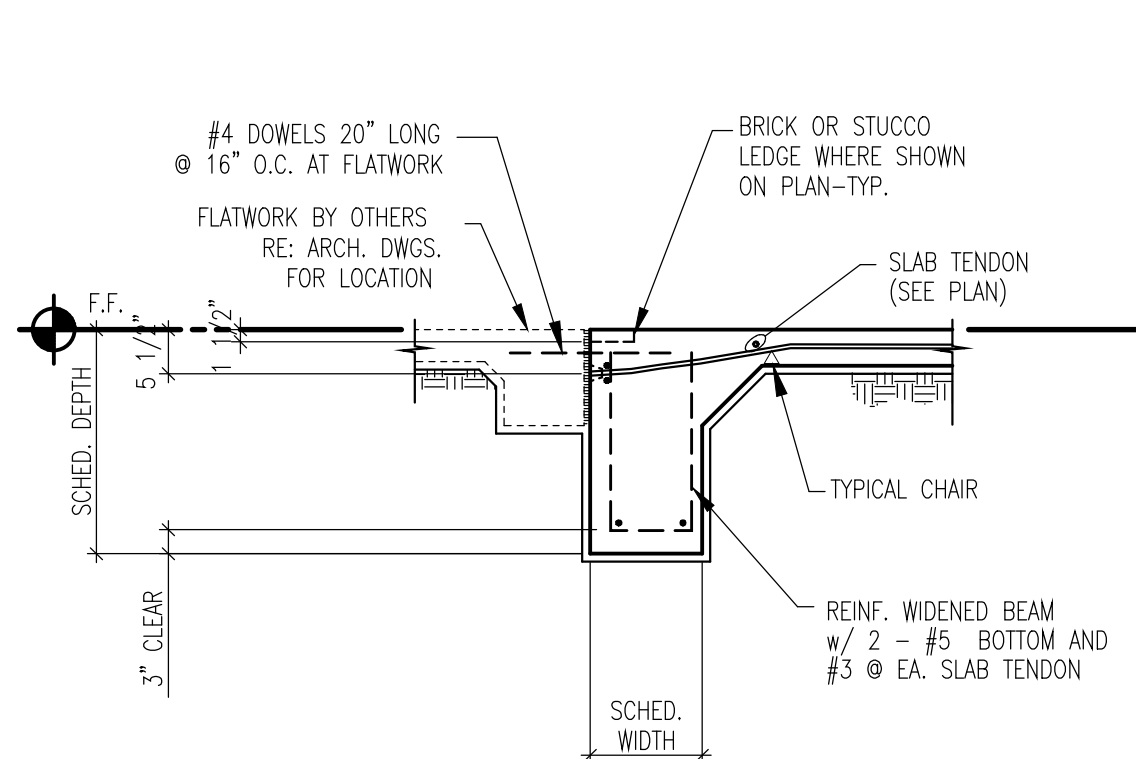
3 WIDENED EXTERIOR GRADE BEAM
SCALE: 1/2"=1'-0"



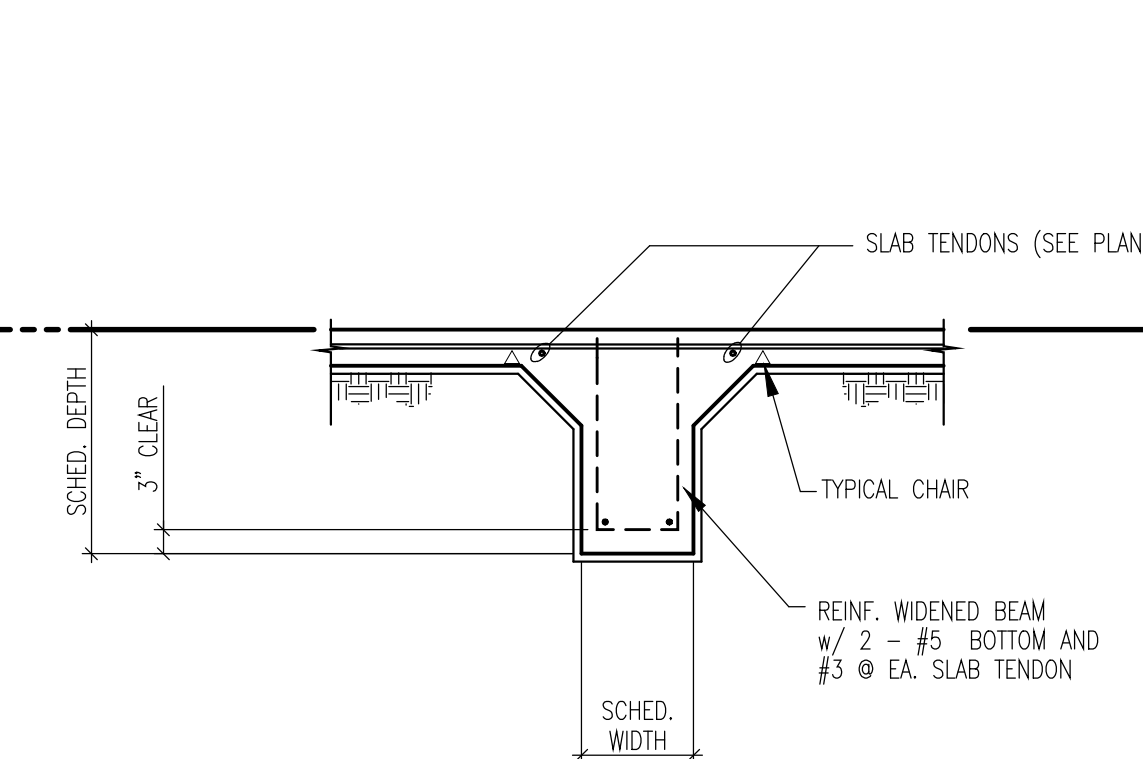
4 WIDENED EXTERIOR GRADE BEAM
SCALE: 1/2"=1'-0"



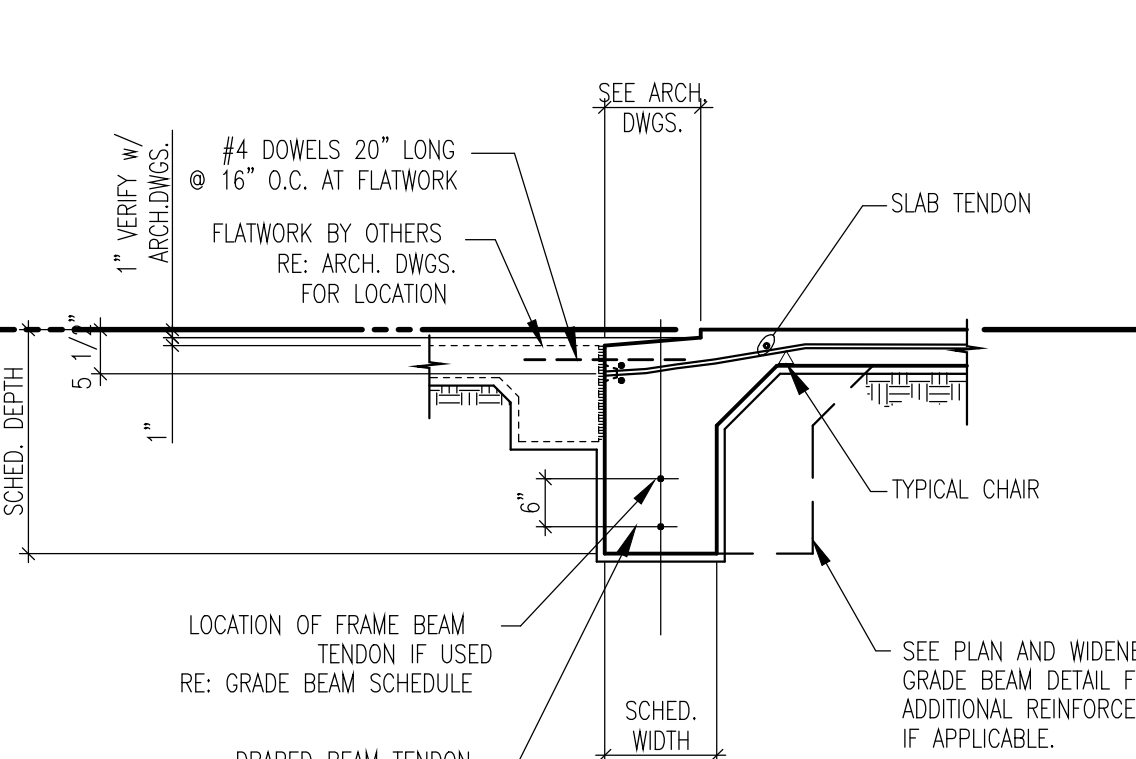
5 WIDENED INTERIOR GRADE BEAM
SCALE: 1/2"=1'-0"



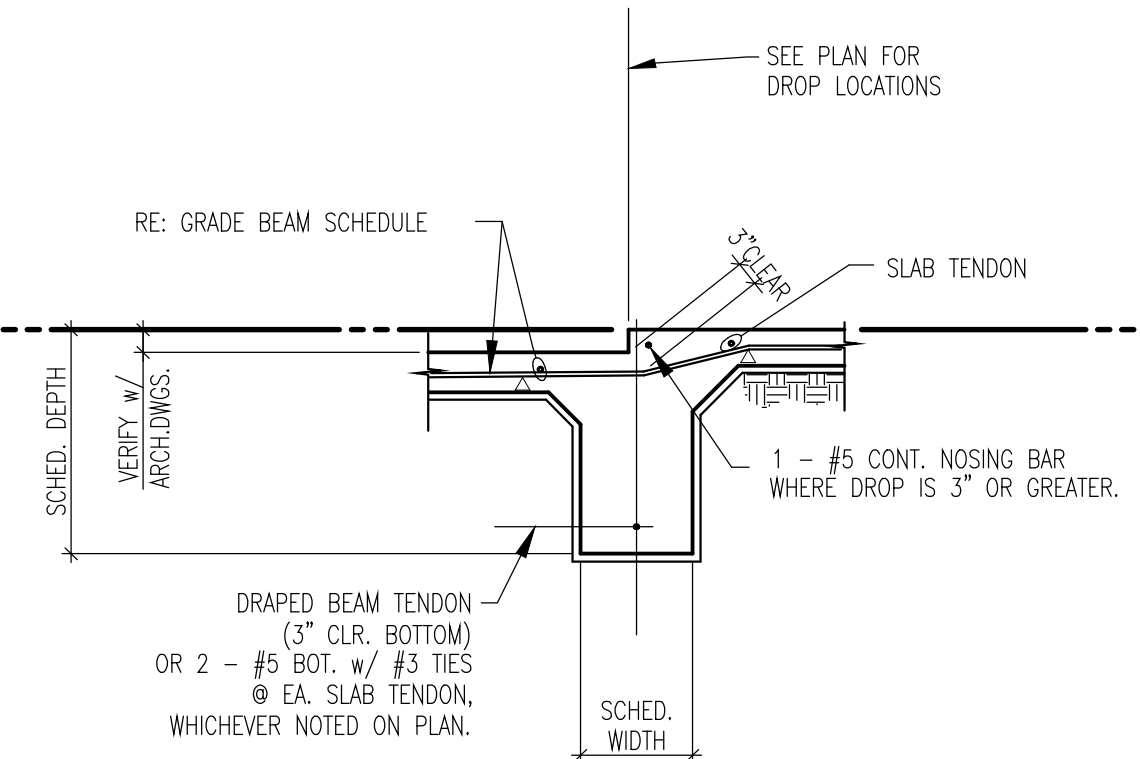
6 TYPICAL REINFORCED GRADE BEAM
SCALE: 1/2"=1'-0"



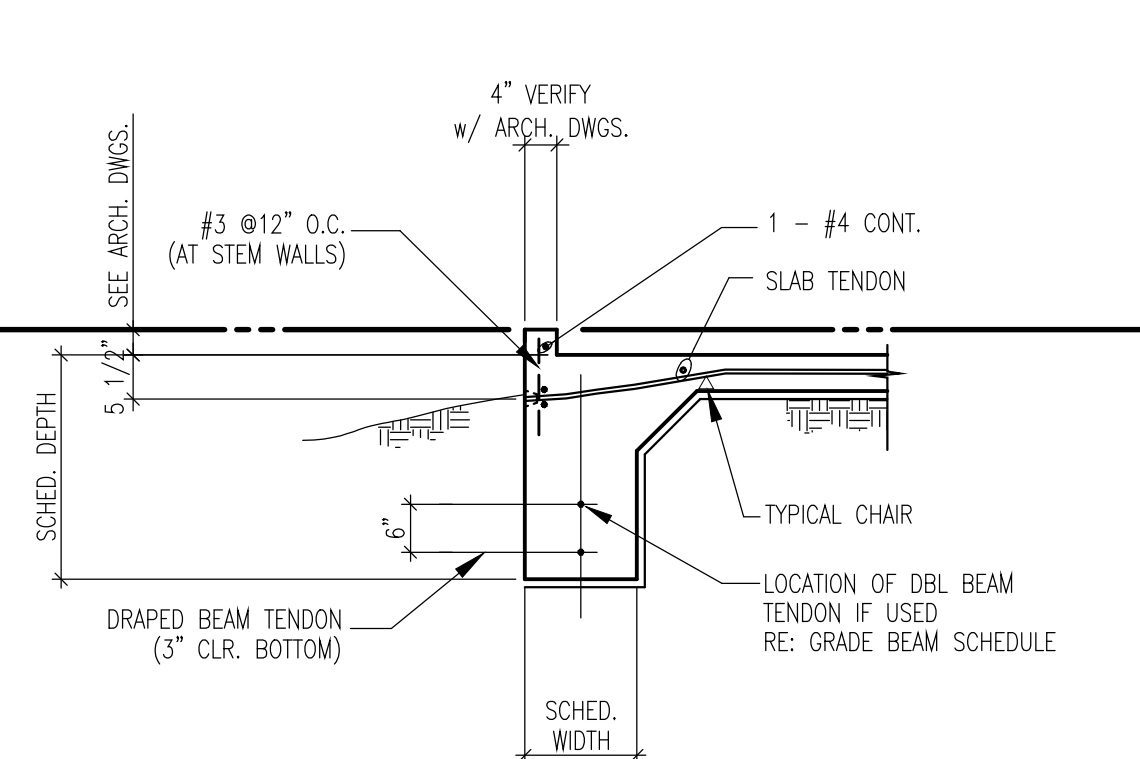
7 TYPICAL REINFORCED GRADE BEAM
SCALE: 1/2"=1'-0"



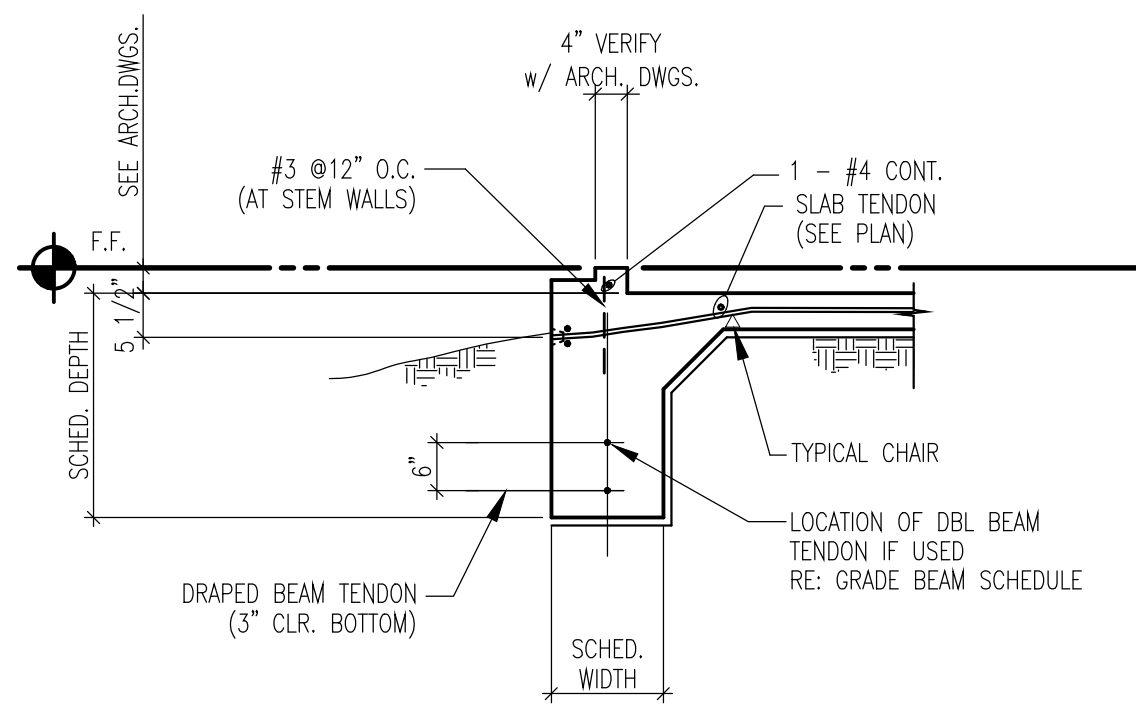
8 GRADE BEAM AT GARAGE DOOR
SCALE: 1/2"=1'-0"



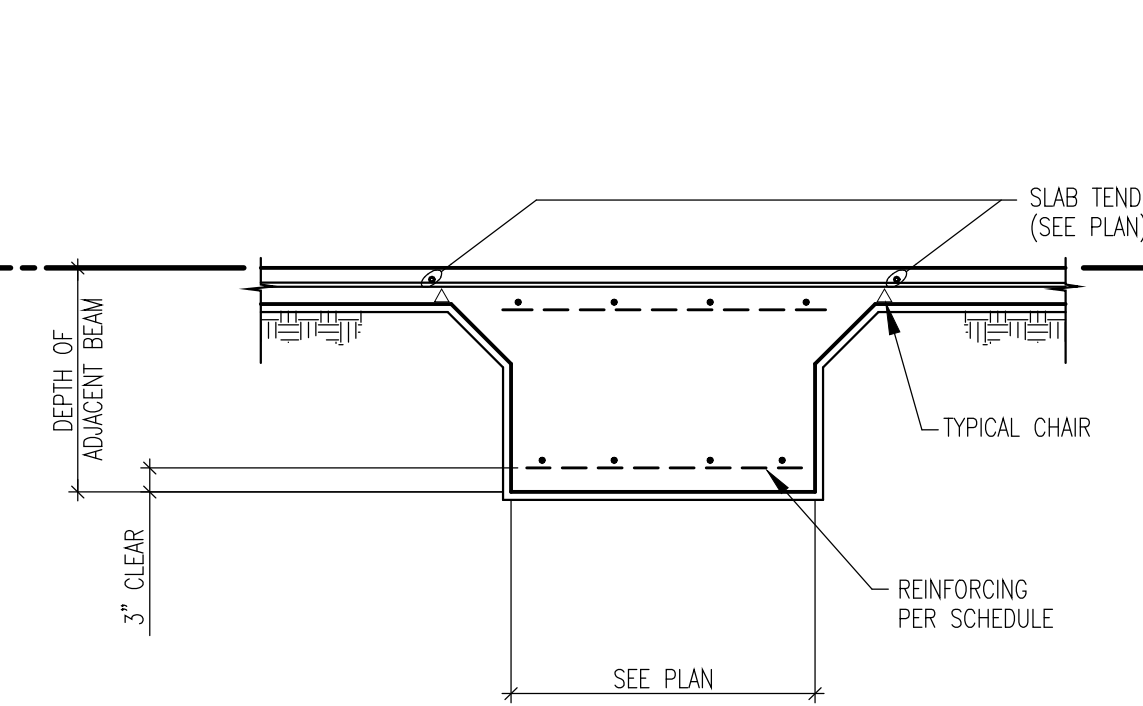
9 GRADE BEAM AT FLOOR DEPRESSION
SCALE: 1/2"=1'-0"



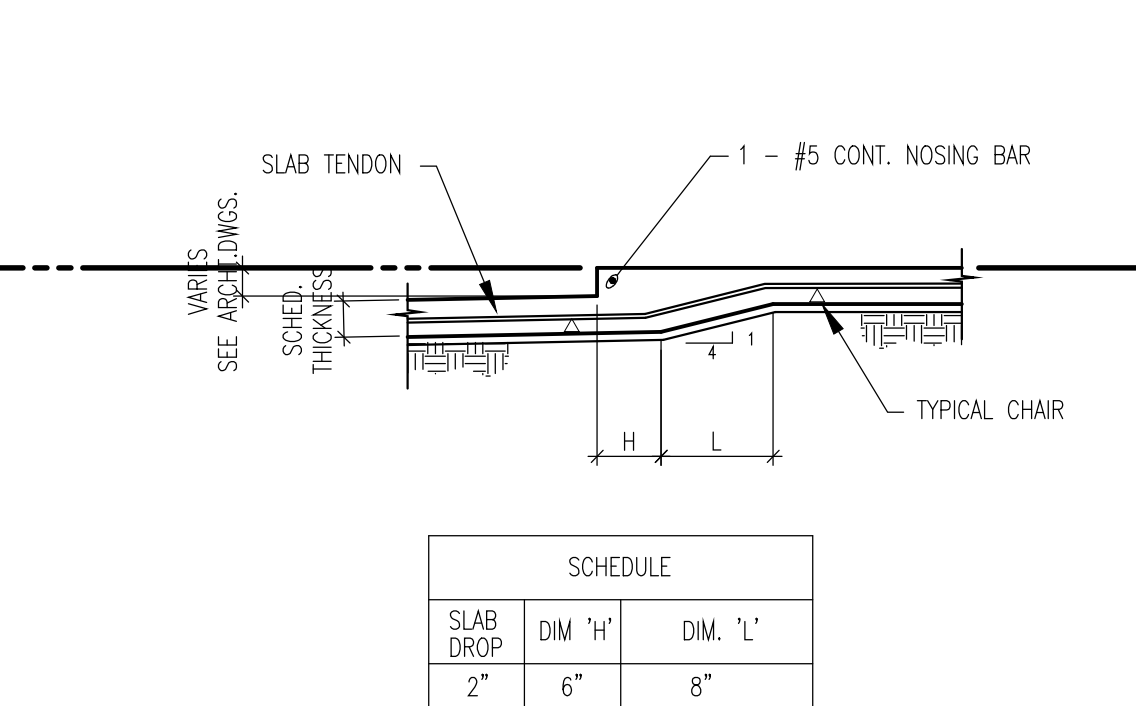
10 EXTERIOR STEM WALL
SCALE: 1/2"=1'-0"



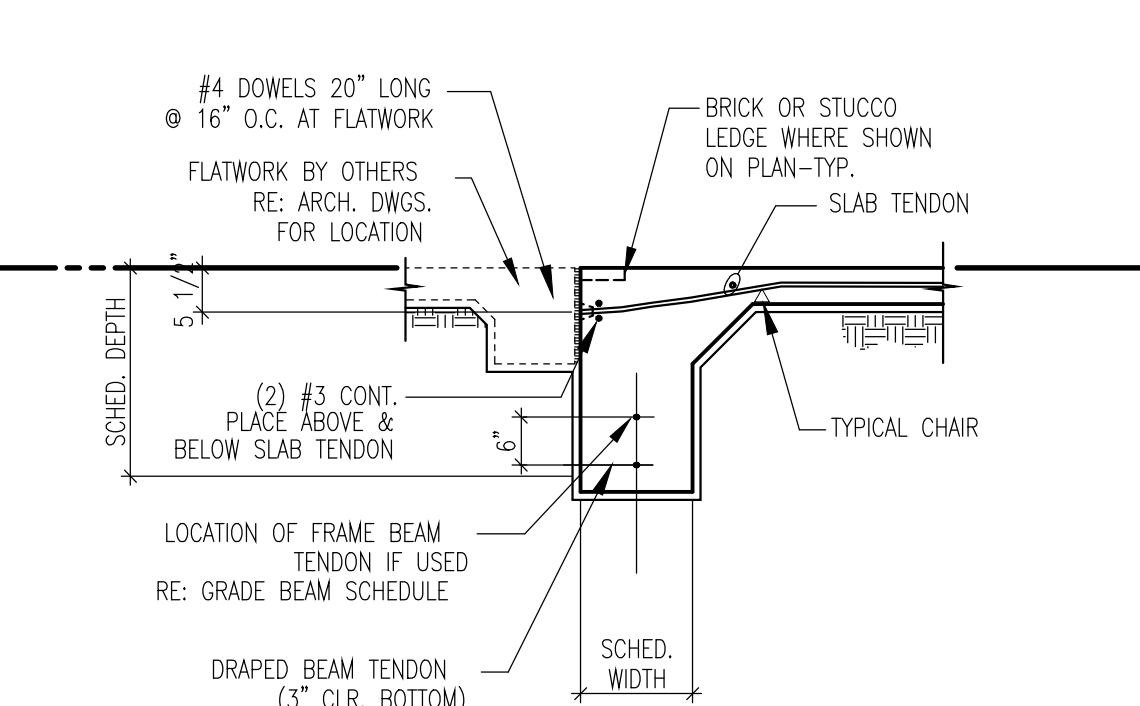
11 STEM WALL @ BRICK OR STUCCO LEDGE
SCALE: 1/2"=1'-0"



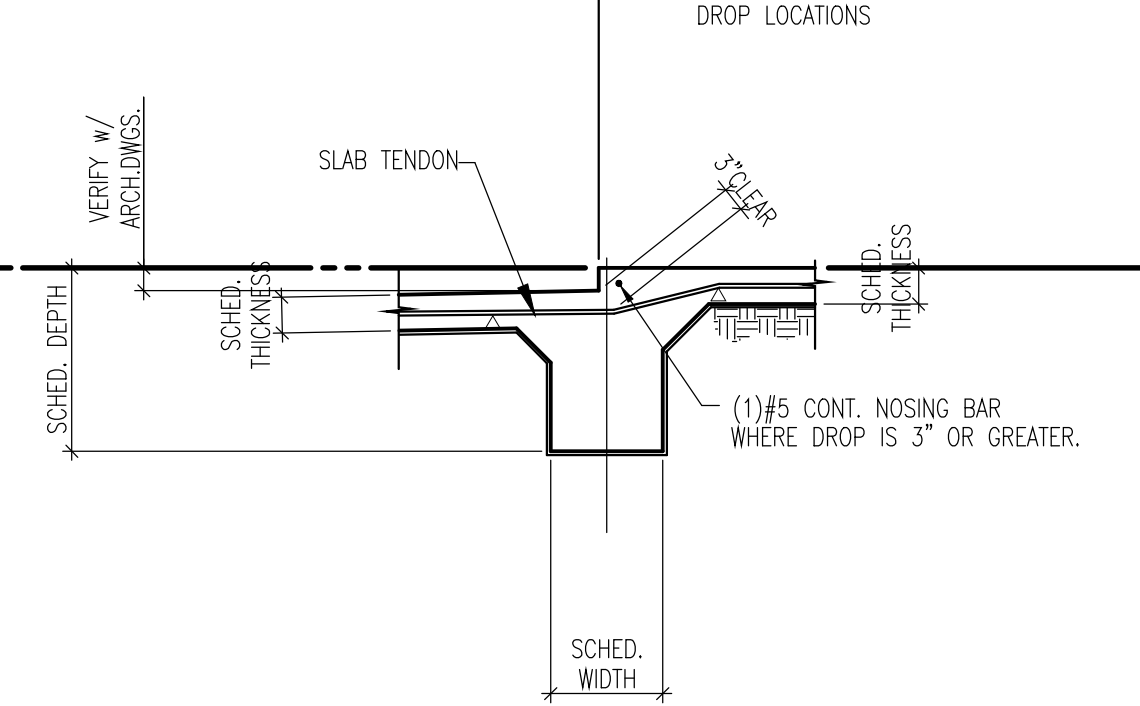
12 SPREAD FOOTING DETAIL
SCALE: 1/2"=1'-0"



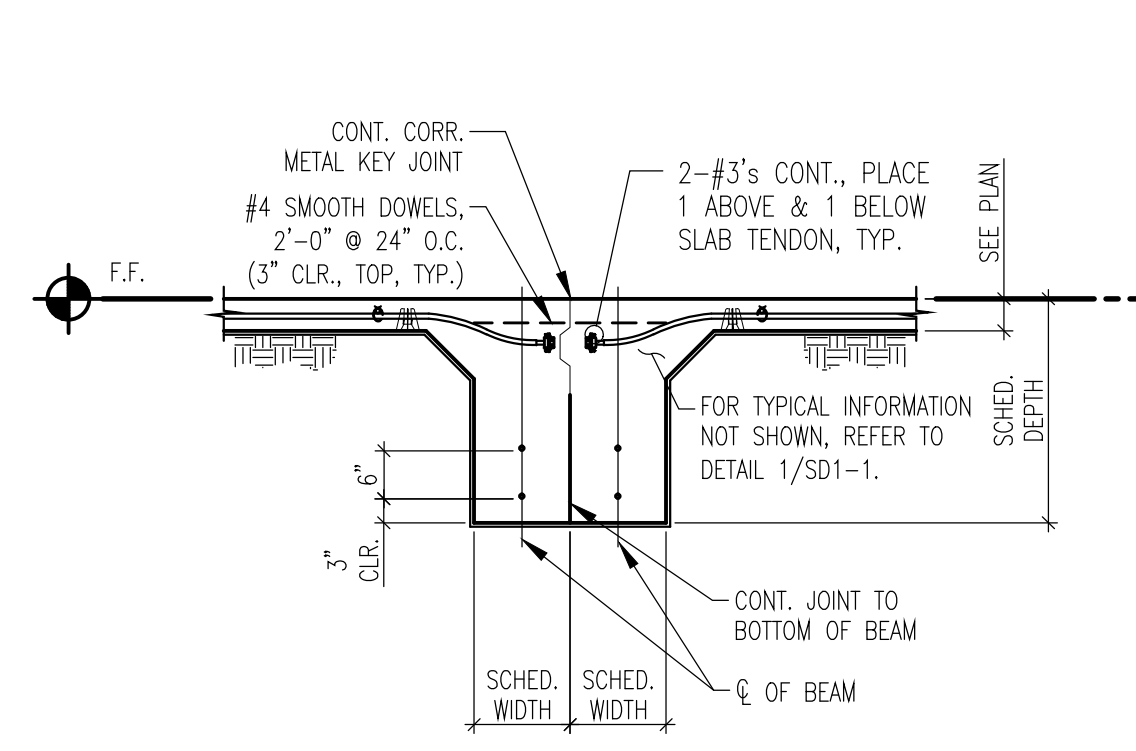
13 SLAB AT FLOOR DEPRESSION
SCALE: 1/2"=1'-0"



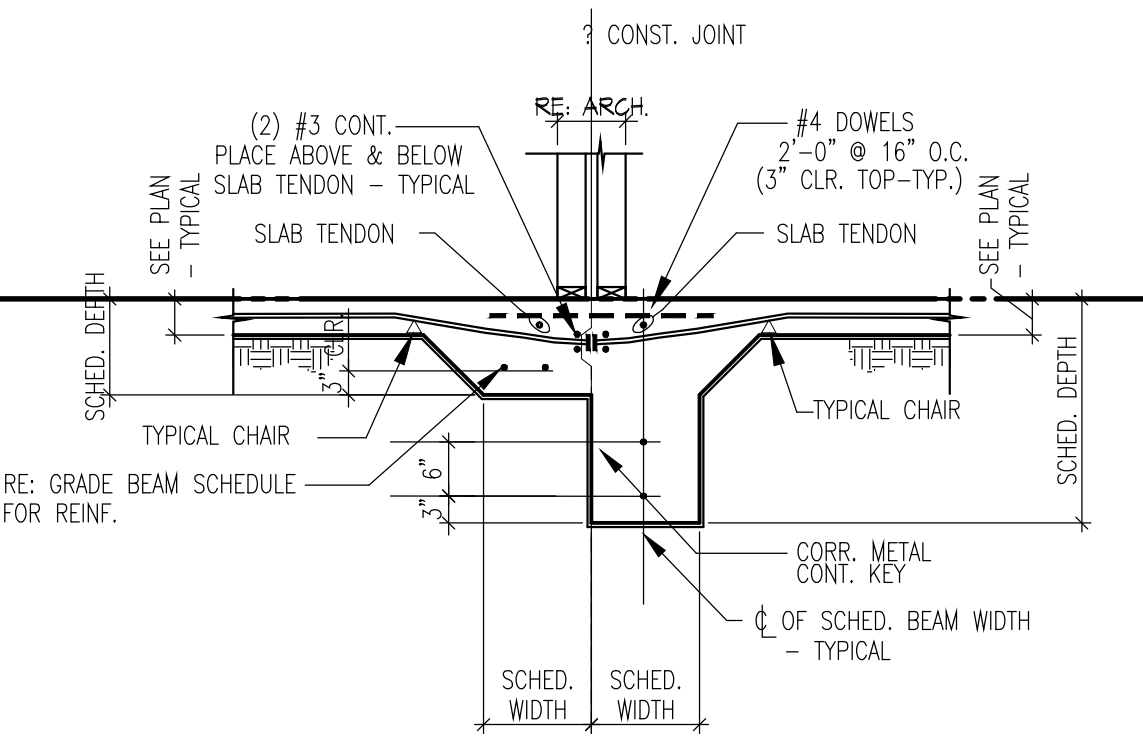
14 EXTERIOR BEAM WITH FLAT WORK
SCALE: 1/2"=1'-0"



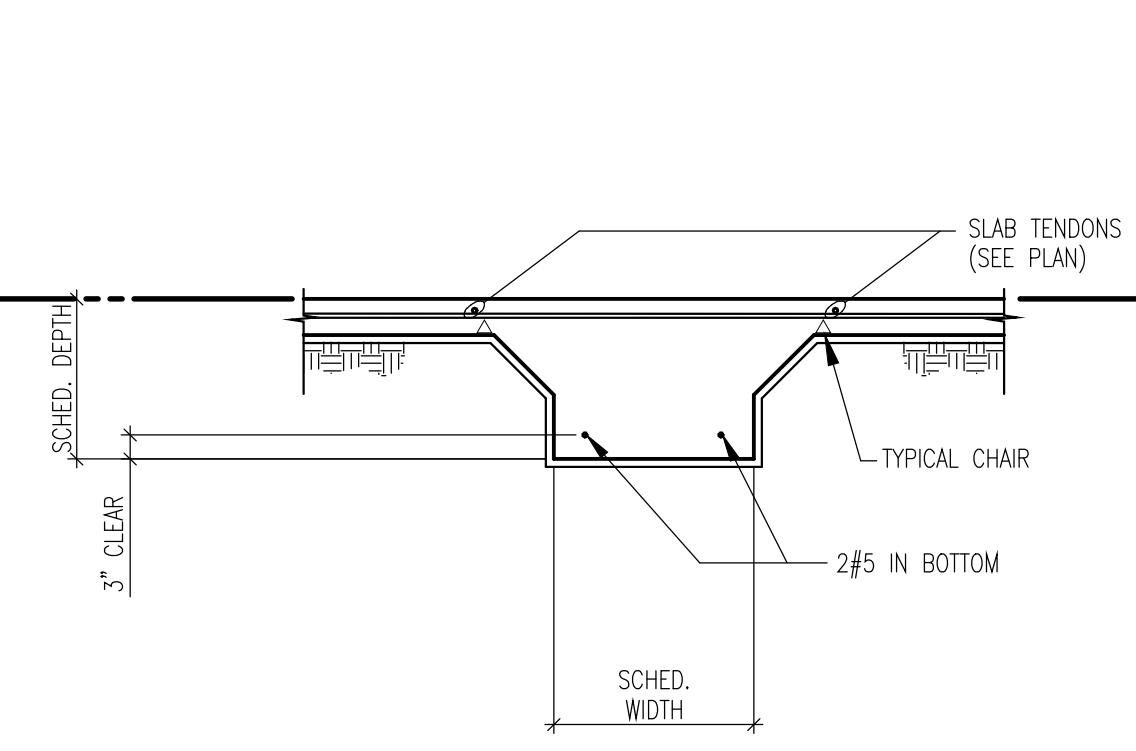
15 THICKENED SLAB AT FLR. DEPRESSION
SCALE: 1/2"=1'-0"



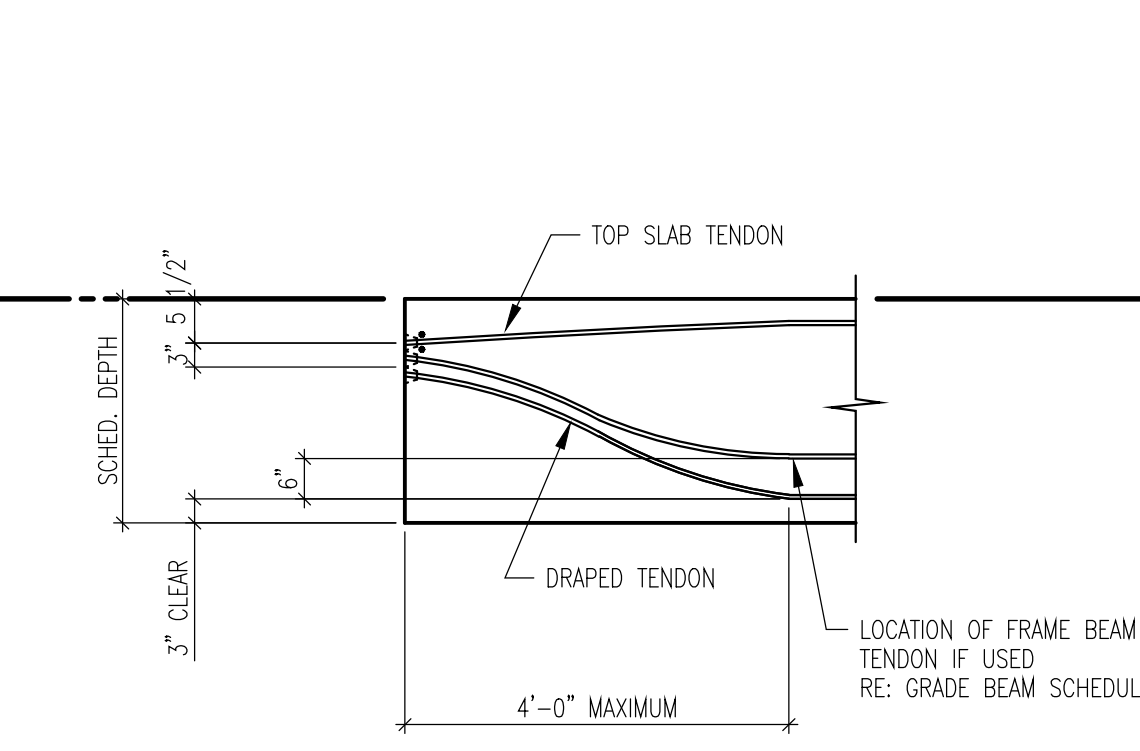
16 DOUBLE BEAM CONSTRUCTION JOINT
SCALE: 1/2"=1'-0"



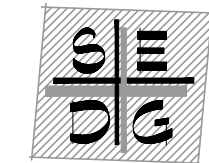
17 CONSTRUCTION JOINT W/ THK. SLAB
SCALE: 1/2"=1'-0"



18 THICKENED SLAB
SCALE: 1/2"=1'-0"



19 DRAPED TENDON BEAM ELEVATION
SCALE: 1/2"=1'-0"



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A Development By [LDG]
Architecture By [KELLY GROSSMAN]

FOUNDATION SECTIONS AND DETAILS

Sheet Title:

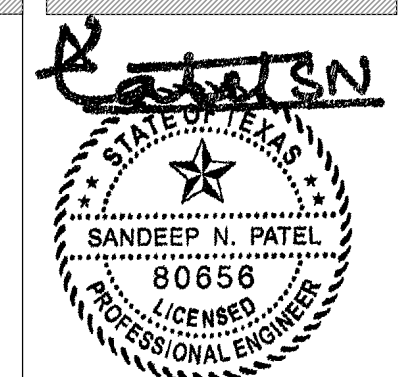
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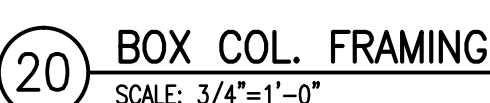
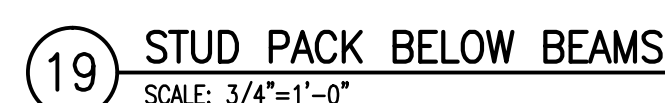
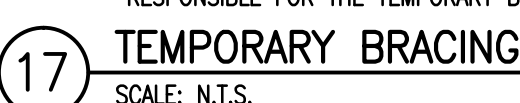
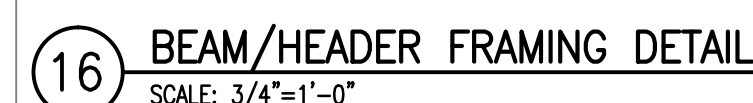
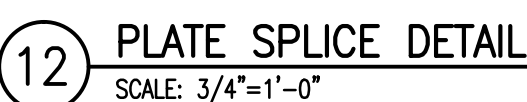
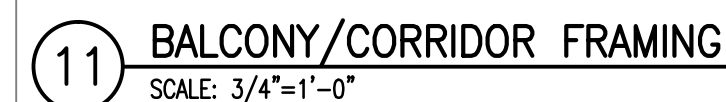
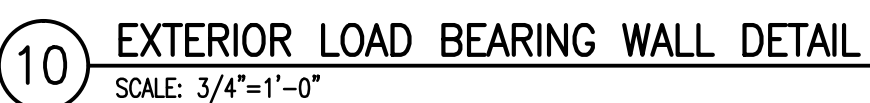
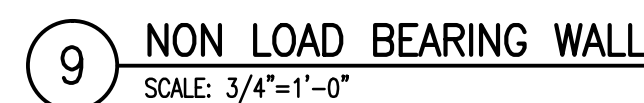
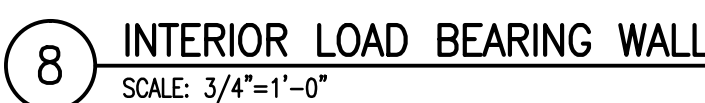
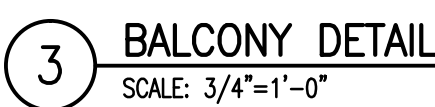
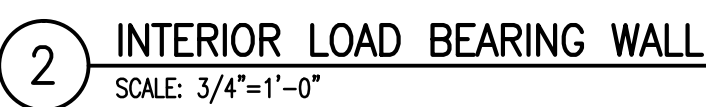
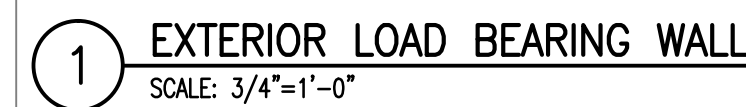
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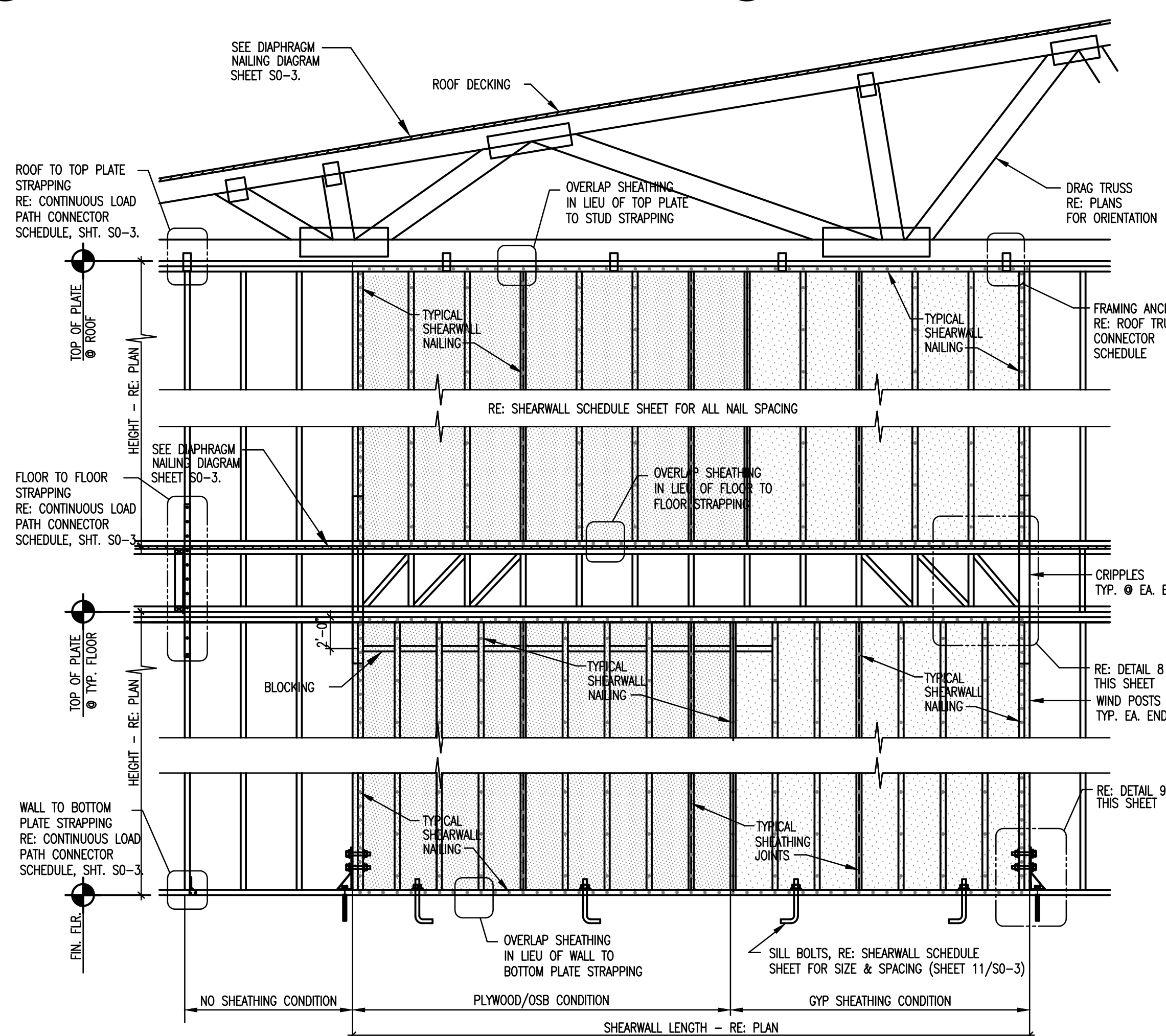
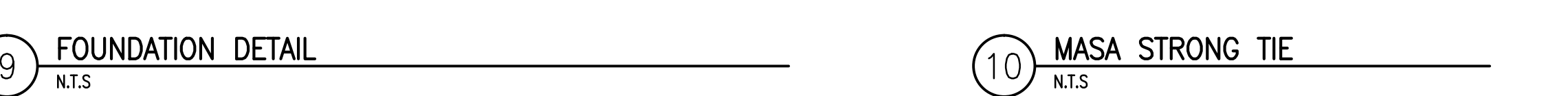
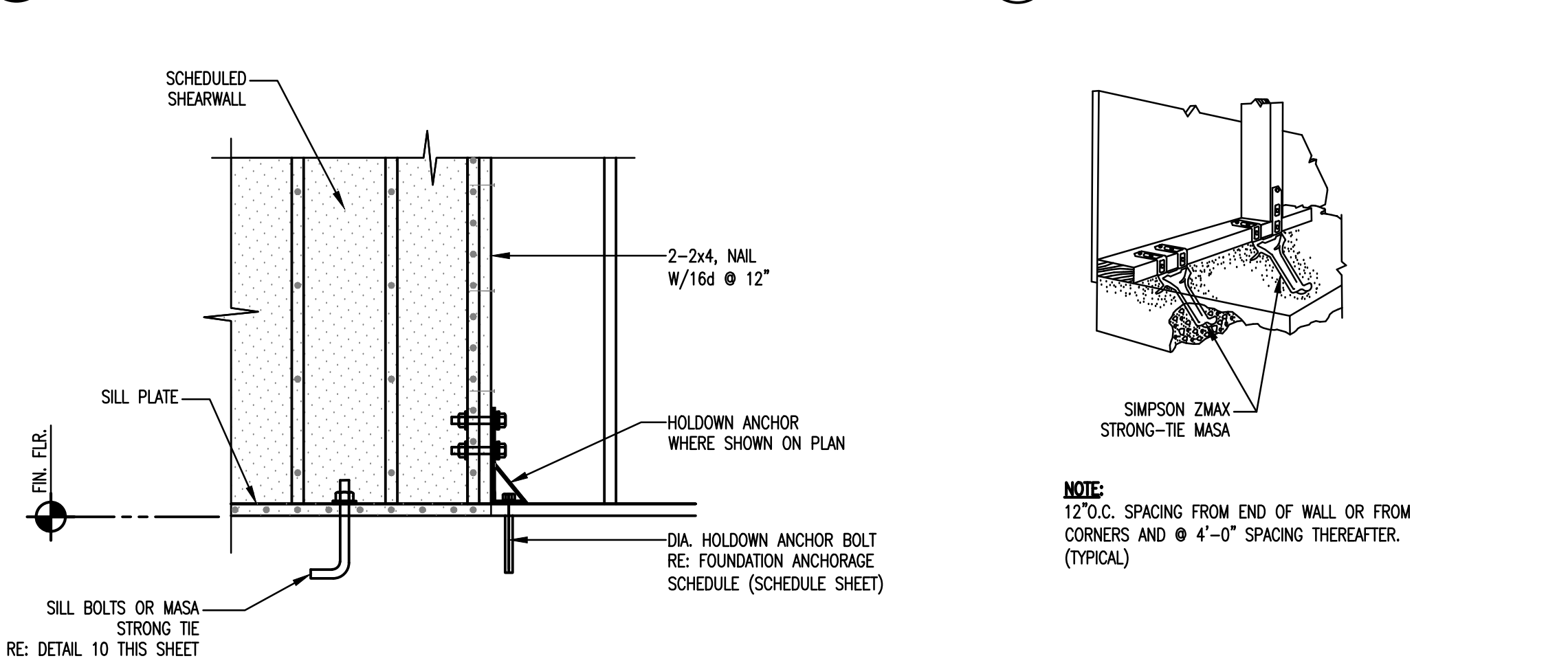
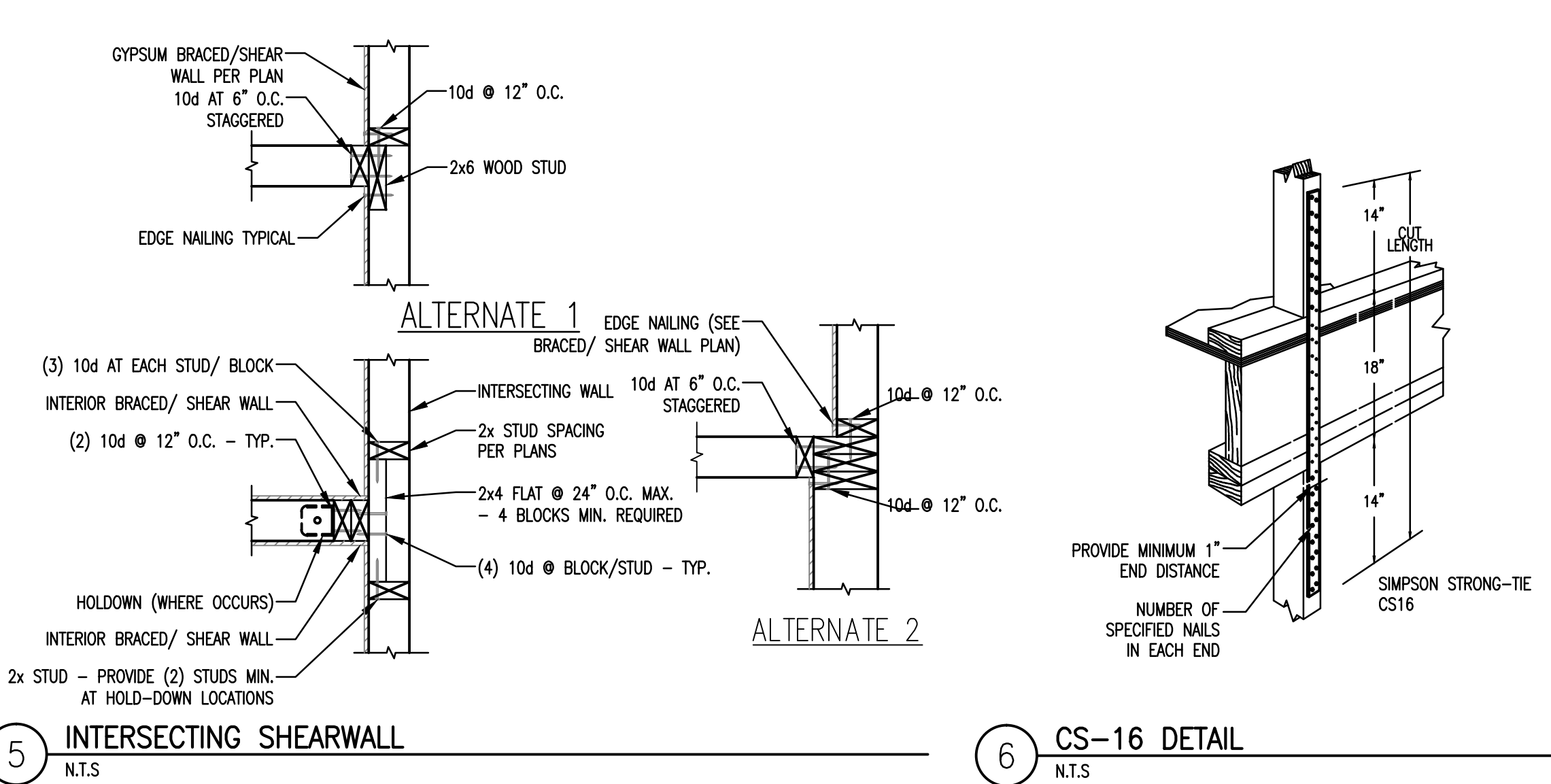
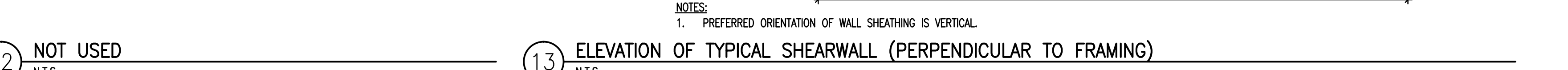
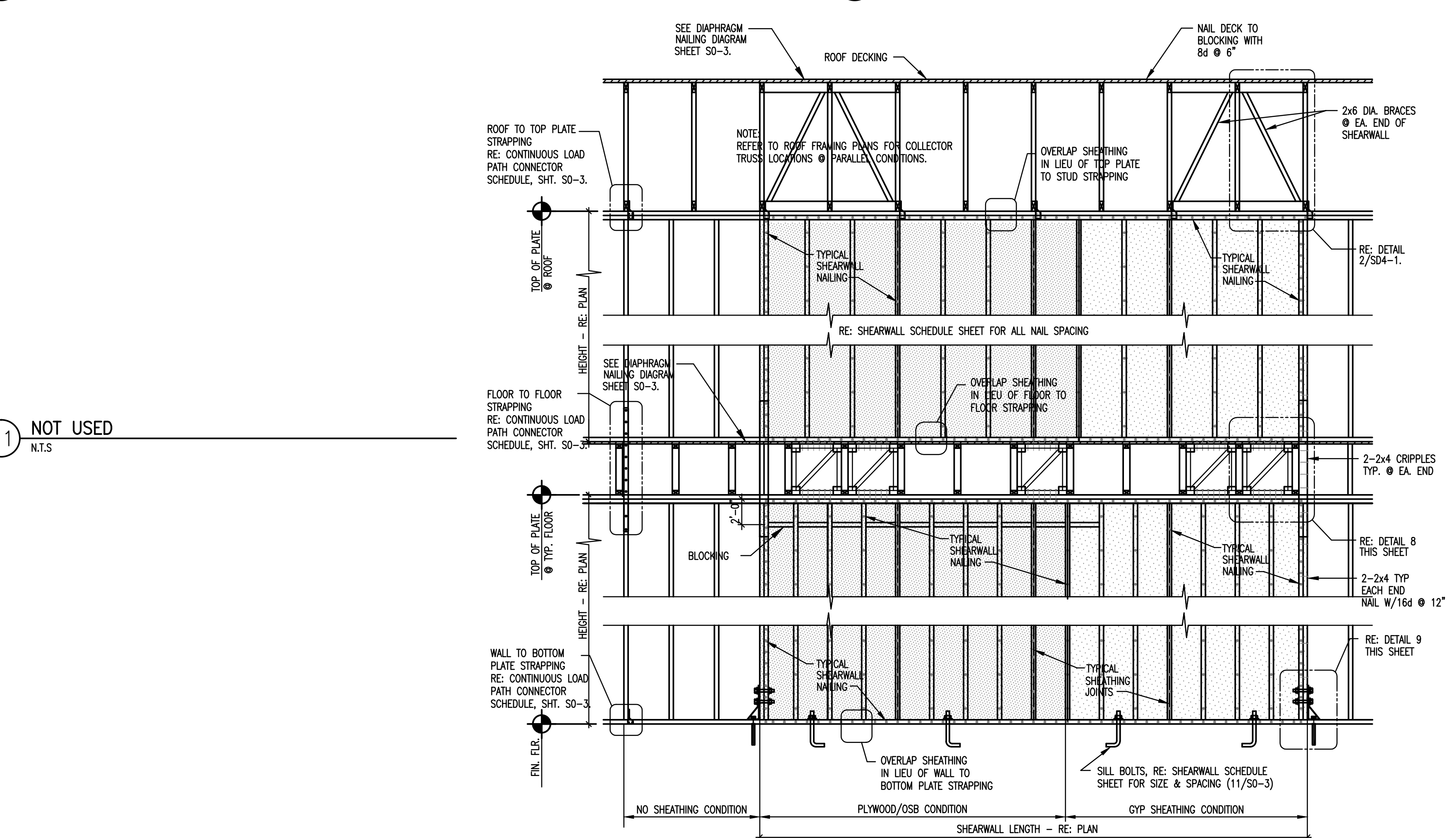
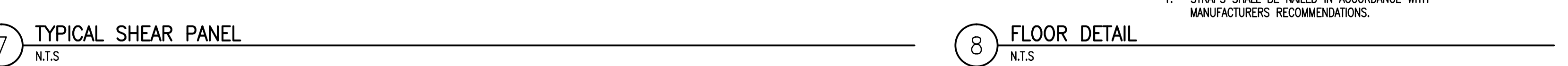
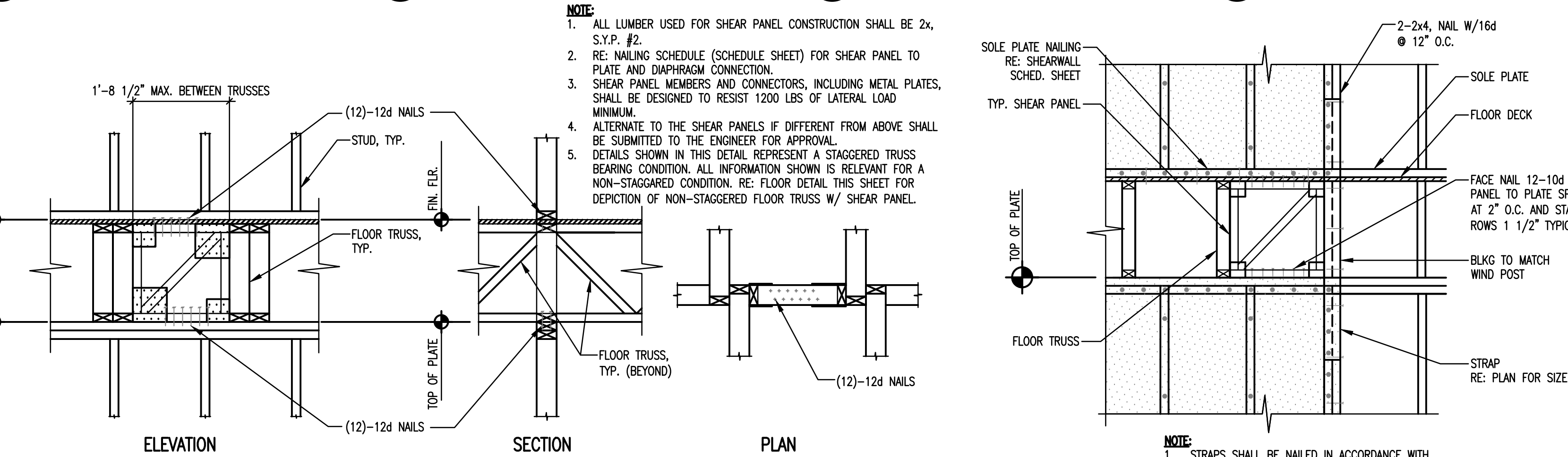
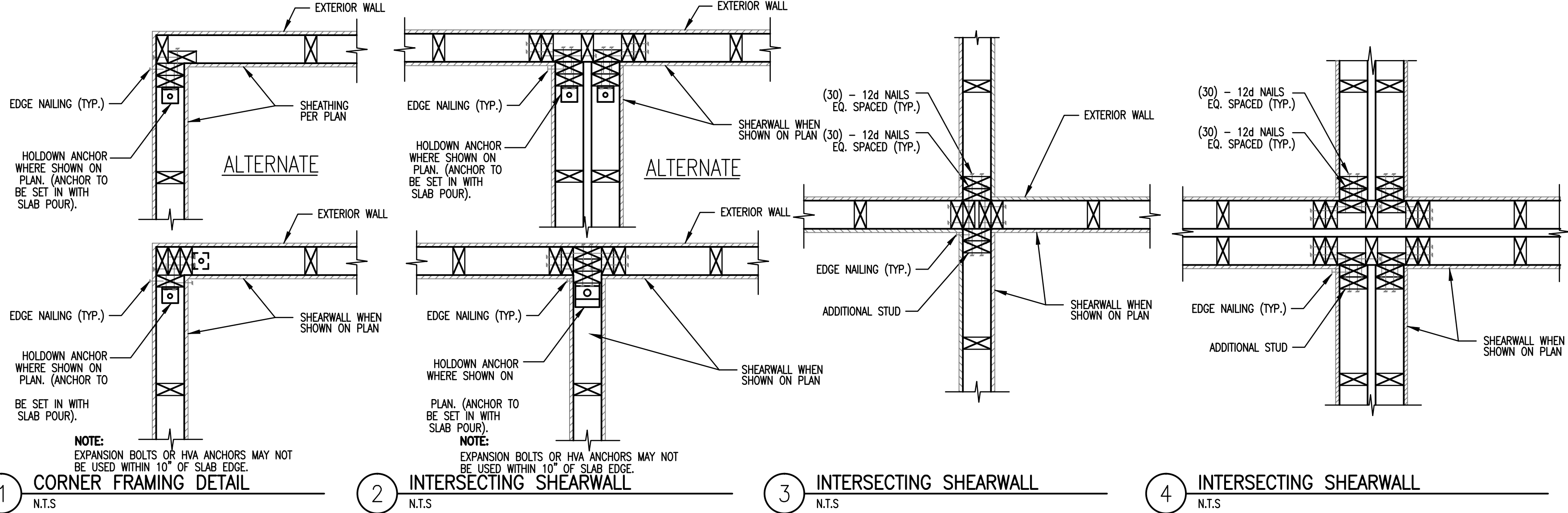
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SHEET NO.
SD1-1
SECTIONS





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MOONLIGHT RANCH APARTMENTS
AUSTIN, TEXAS
A Development By [LDG]
Architecture By [KELLY GROSSMAN]

SHEET TITLE: SHEARWALL SECTIONS AND DETAILS

Rev.	Description	Date

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Checked By: DW/ZA
Drawing Scale: As Noted
Project No. 136-091

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SANDEEP N. PATEL
80656
PROFESSIONAL ENGINEER

06/10/2019
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